

Bulletin of the Seismological Society of America

Studies of mechanisms for water level changes induced by teleseismic waves

--Manuscript Draft--

Manuscript Number:	BSSA-D-13-00213
Article Type:	Article
Section/Category:	Regular Issue
Full Title:	Studies of mechanisms for water level changes induced by teleseismic waves
Corresponding Author:	Yan Zhang, Ph.D. Institute of Geology and Geophysics, Chinese Academy of Sciences Beijing, CHINA
Corresponding Author's Institution:	Institute of Geology and Geophysics, Chinese Academy of Sciences
Corresponding Author E-Mail:	eve_041744@163.com
Order of Authors:	Yan Zhang, Ph.D. Li-Yun Fu Fuqiong Huang Lian-feng Zhao Yuchuan Ma Bo Zhao Benxun Su
Abstract:	<p>The Ms 8.0 Wenchuan earthquake of May 12, 2008 induces large-amplitude water level changes at intermediate and far fields (epicentral distance >1.5 fault rupture length) in Chinese mainland. Although many hydrologic changes induced by teleseismic waves have been reported, the mechanisms responsible for the changes still remain unclear. We invoke Skempton's coefficient B and effective pressure in this paper to explain those co-seismic water level changes documented in the intermediate and far fields. The most used "enhanced permeability with a rapid redistribution of pore pressure induced by removing loose particals from fractures by teleseismic waves" can not be applied to explain all those coseismic water level changes in this study. From our research we find some of those abrupt coseismic water level changes, for which the variation of the co-seismic water level, and the effective pressure preserve consistent (all increase or all decrease) are found to favor the consolidation (porosity decrease) / dilatation (porosity increase) induced by the shaking of teleseismic waves. Most of those wells have relatively high permeabilities attributing to the shales in the aquifer lithologies. While the other part of those coseismic water level changes (the variation of the co-seismic water level keeps inconsistent with the variation of effective pressure), can be explained with the enhanced permeability with a rapid redistribution of pore pressure, which is caused by fracture clearing or overcoming the capillary entrapment in porous channels of the aquifer induced by the shaking of teleseismic waves (most probably long period surface waves). Most of those wells stay in basins or hollows, this kind of terrain inclines to lead to heterogeneous pore pressure in close proximity.</p>
Author Comments:	<p>Although many hydrologic changes induced by teleseismic waves have been reported, the mechanisms responsible for the changes still remain unclear. We invoke Skempton's coefficient B and effective pressure in this paper to explain those co-seismic water level changes documented in the intermediate and far fields. The most used "enhanced permeability with a rapid redistribution of pore pressure induced by removing loose particals from fractures by teleseismic waves" can not be applied to explain all those coseismic water level changes in this study. From our research we find some of those abrupt coseismic water level changes, for which the variation of the co-seismic water level, and the effective pressure preserve consistent (all increase or all decrease) are found to favor the consolidation (porosity decrease) / dilatation (porosity increase) induced by the shaking of teleseismic waves. Most of those wells</p>

	have relatively high permeabilities attributing to the shales in the aquifer lithologies.
Suggested Reviewers:	Chi-yuen Wang chiyuen@berkeley.edu
Opposed Reviewers:	Yaowei Liu He has a conflict with one of the auther

Ref.: Ms. No. BSSA-D-12-00360R2

Studies of mechanism for water level changes induced by teleseismic waves

Bulletin of the Seismological Society of America

Dear Yan Zhang,

Your paper has been reviewed for publication in the Bulletin. I enclose two reviews by anonymous referees who have raised serious concerns. Reviewer #3 did not review the first draft of the manuscript but has similar criticisms to the two reviewers who reviewed the first draft. These are appended below. The editorial board has evaluated the reviews and has found the paper to be unacceptable for publication. First, the English is still poor and the paper lacks structure/organization, although you were given significant input from two reviewers and an associate editor on how to improve the grammar and organization of the manuscript. Second, you do not appear to address the technical concerns of the reviewers and associate editor in your revision. I believe the Editorial report and reviews adequately explain the reasons for this decision and I hope you find them useful.

Thank you for your interest in the Bulletin.

Sincerely

Diane I. Doser, PhD

Editor-in-Chief

Reviewers' and associate editor's comments:

Associate Editor:

Please see the comments from the reviewers. Try to incorporate well log data to support your view. You may need to redo this work and resubmit it.

Reviewer #2: Attached. (note this reviewer reviewed version 1 of the manuscript as well as your revised manuscript)

The authors have addressed the issues which have asked by the other reviewer. But still I feel scientifically the paper is ready to publish whereas grammatically (including structure of the sentences) it should be rechecked (The authors may take help of someone whose native language is English).

Answer: According to the suggestion and comments of reviewer 3, we have modified the paper significantly, so as to explain the mechanism much more clearly, see those modifications annotated in green colors.

e.g. Line:106 Page 5

“The detailed borehole columnar diagrams (borehole 107 columnar diagram of well b, g, h, i, and j cannot be found) are not **show** in this paper those information obtained from the borehole columnar diagrams together with the aquifer lithology are **show** in Table 1.”

Answer: According to the suggestion of reviewer #3, we added those borehole columnar (well lithologic logs), so we have already deleted this sentence.

e.g. Line:236 Page 10

“The local geological structure of each well is important (**Table 1**), we **find** that most of those wells in which.....”

Answer: We have modified the sentence into two sentences, See Line 272.

e.g. Line:240 Page 11

“.....will not easily to be incurred, then the energy of shaking may be inclined to induce the fracture clearing (unclogging)....”

Answer: We have changed the content because of the modification suggested by the reviewer #3. Please see Line 271—276.

e.g. Line:404 Page 17

“After comparison, generally we may use the seismograms of 4 national stations to analyze the corresponding water....”

What do you mean by "may"? You have used already. Isn't it?

Answer: Yes, your suggestion is good. However, after the suggestion of reviewer 3, we estimate the quality of those seismograms, finally we only can use 2 of those seismograms, so we still use “may”. The content has been changed a lot, see Line 505—552.

Reviewer #3: The authors of this draft show water level changes due the Wenchuan Earthquake, recorded in several wells in mainland China, at distance too far to attribute these changes to poroelastic response to static stress changes. They claim these changes is due to a variation in Skempton coefficient "B", rather than changes in other poroelastic coefficients or permeability. This change in B is related to a theory of "consolidation/dilation".

The paper is difficult to read, first because of language issues, second because of the poor construction of the discussion.

The consolidation/dilation theory is quite unclear, partially because of the lack of equation. Also, during their discussion, it is unclear whether the medium is fractured or porous. The relationship between porosity, elastic modulus and porosity may be quite different in these cases. The author do not take time to discuss their raw data, and comment the order of magnitude of their results. Quality control of data and analysis should be discussed in a first part of the discussion, not left to the discussion at the end of the paper.

Answer: These are good suggestions, we have done an enormous modification (see those annotated green color parts), including the construction of the discussion, the order, and the analysis of well logs. Especially, your suggestion to use equations is a terrific idea, and we summarize the variation of effective pressure ($P_{eff} = P_c - P_p$) in two ways, which

can help us to analyze the mechanism much more clearly. See: Line 209—234

There are several points which need to be clarified.

- Does the poroelastic theory used by the authors apply to the formation in their wells? For instance, lithological logs shows shales and crystalline rock. The first rock may display substantial anisotropy or a fractured network rather than a porous network. Previous reviewers asked for more log data to clarify this point, but the authors did not reply to their request.

Answer: We have added those logs, please see Figure 8.

However, there are so much wells has the fractured aquifer, and poroelastic theory is an ideal theory, it suppose the medium to be linearly elastic isotropic porous medium, Fluid saturated crust behaves as a poroelastic material to a good degree of approximation. Even if the rock is anisotropy or a fractured network rather than a porous network, we suspect that the isotropic and homogeneous poroelastic theory we used is the best available approximation. (We have consulted several experts in this research region, and they all agree with this viewpoint). Set an example: There are large distances between stations and the epicenter, and there are lots of faults (so the medium is not uniform). The Okada dislocation model (Okada, 1992; Lin and Stein, 2004; Toda et al., 2005) is based on the assumption that the whole land is isotropic and homogeneous. Therefore, there may be some differences between the calculated volume strain change and the real value, however, till now, most of us still use the Okada dislocation model to calculate the volume strains, and it might be the most useful means. We also add this discussion into the conclusion part, see Line 602—612.

As indicated by this reviewer, those log analysis are very useful. Especially the aquifer with shales, which may display a (fractured) high permeability nature, and this help us to analyze the mechanism much more deeply. See: Part “4.3 Well logs and permeability” Line 375–408 (and also Abstract, Introduction).

- The Skempton coefficients are very small for many wells (<0.1). At the recorded depths, we expect fully saturated rocks, and Skempton coefficient are expected to be larger than 0.5 (see final tables of Wang, 2010, citation of l. 585). If the medium is unsaturated, the authors should state that.

Answer: This may be attributed to the value of the shear modulus G (see Zhang and Huang (2011), since we lack the in-situ G values, we investigate the geology of each well and referred to the rock mass mechanism (Liu and Tang, 1998), using the dynamic elastic modulus and dynamic Poisson's ratios to estimate the ranges of the dynamic shear modulus of those matrix rocks (according to the formula $G = \frac{E}{2(1+\sigma)}$), and to choose the approximate mean values (Table 1)). [See Table 1 (Shear modulus G* see Yan Zhang and Fuqiong Huang (2011)].

Below is Table 1 of Zhang and Huang (2011)

Table 1
Dynamic Deformation Parameters of Rocks

Rock	Dynamic Elastic Modulus E (GPa) *	Dynamic Poisson's Ratio σ *	Dynamic Shear Modulus G (GPa)	Rough Value of Dynamic Shear Modulus (GPa)
Sandstone	5.3 ~ 37.9	0.20 ~ 0.22	2.17 ~ 15.79	8
Graniton	63.4 ~ 114.8	0.20 ~ 0.21	26.20 ~ 47.83	36
Quartzite	20.4 ~ 76.3	0.23 ~ 0.26	8.10 ~ 31.02	20
Limestone	12.1 ~ 88.3	0.24 ~ 0.25	4.84 ~ 35.60	20
Gneiss	76.0 ~ 129.1	0.22 ~ 0.24	30.65 ~ 52.91	40
Granite	37.0 ~ 106.0	0.24 ~ 0.31	14.12 ~ 42.74	28
Whinstone	53.1 ~ 162.8	0.10 ~ 0.22	21.76 ~ 74.00	48
Diorite	52.8 ~ 96.2	0.23 ~ 0.34	19.7 ~ 39.11	30
Psephite	3.4 ~ 16	0.19 ~ 0.22	1.39 ~ 6.723	4

*See Liu, Y. R., and H. M. Tang (1998), p. 112.

In addition, Sil and Jeffrey (2006) (obtained an average Skempton's coefficient B value of 0.02) and Chadha *et al.* (2008) have obtained the similar low value of B , which indicate that the wells are not perfectly confined and the aquifers are highly permeable. So we indicate: the assumption of undrained condition may not be strictly meet with. We have discussed this in the conclusion part, [See line 606—616.](#)

- The authors focus on the change in Skempton coefficient, dismissing any change in other coefficients. For instance, as cited in line 141, Berryman and Wang (2001) show a large variation in bulk modulus K_u in their data. Remember, that the tidal amplitude of water level changes is controlled by $B \times K_u$. I don't understand why the author cite the work done on bone by Theo H Smit, Jacques Huyghe and Stephen C. Cowin (note that the authors cited these authors by their first name): in this paper, they discuss the dependency of the coefficient on porosity. Do the author think that porosity is changing due to shaking? In that case, it should be clarified when discussing the mechanism, because from line 352, I thought it did not.

Answer: Please see part 3.1 "Assumptions of shear modulus and Poisson's ratio and the calculation of Skempton's coefficient B "

We have use the previous results from the former researchers to justify that, compared with the pre- and post-earthquake Skempton's coefficient B , the shear modulus and the poisson's ratio can be neglected. [See Line 116—146.](#)

We cite the work of Theo *et al.* (2002) is to clarify that "compared to the variations of Skempton's coefficient B , the change of the undrained poisson's ratio can be neglected before and after the earthquake." [See Line 116—130.](#)

As show in [Line 203.](#) Permeability will increase/decrease, which is mostly related to the increase/decrease of porosity (Xue, 1986). So, in the mechanism analysis (which we have modified a lot), we do discuss about the porosity together with the permeability, both of which change in accordance. [Line: 190—447. \(" Mechanism analysis"\)](#)

- The description of the consolidation/dilation model is very confusing. To be improved, it would be helpful to get a set of equations and a sketch precisising the conceptual model of the medium (is it fractured? porous ?). This would replace the hand waving of lines 199-204. It would provide also an expected range for the linear relationship found between changes in effective pressure and in B . This theoretical framework would be helpful, because they do not provide any citation or evidence for why B would increase with effective pressure (the experiments of Blocher 2009 show a negative trend, but with effective pressure starting at 5MPa, and the apparent B changes in the study may be also contaminated by permeability or K_u changes).

Answer: Yes we also feel the description is confusing, your suggestion to use equations is a terrific idea, and we summarize the variation of effective pressure ($P_{eff} = P_c - P_p$) in two ways: (P_c confining pressure, P_p pore pressure, and P_{eff} effective pressure), which can help us to analyze the mechanism much more clearly. [See. Line 209—232. \(Table](#)

4

As indicated by this reviewer, those log analysis are very useful. Especially the aquifer with shales, which may display a (fractured) high permeability nature, and this helps us to analyze the mechanism much more deeply. See: Part “4.3 Well lithologic logs and permeability” [Line 375-408](#) (and also Abstract, Introduction)

However, one thing needs to be clarified: we say those co-seismic water level changes fit to be explained with the consolidation/dilation model, and those consolidation/dilation are induced by teleseismic waves: Permeability decrease is corresponding to the porosity decrease, and which indicates the consolidation of the aquifer, this mechanism is much similar with the mechanism proposed by Liu and Manga (2009). [See Line 303—318](#): “From the laboratory experiment, Liu and Manga (2009) find that: in general, permeability/porosity decreases after shaking. They measured the evolution of permeability in fractured sandstone in response to repeated shaking under undrained conditions, and set the frequency and amplitude of the imposed shaking to be representative of those that cause distant hydrological responses. As they explained: Dynamic strains cause time varying fluid flow that can redistribute particles within fractures or porespace, and can allow particles to move away from regions where they hold pore spaces open, and are expected to accumulate and get trapped at the narrowest constrictions along flow paths, and hence allow a consolidation (contraction) of the sample, which can lead to a higher coupling between the stiff rock matrix and the fluid. Their result just supports our mechanism analysis. It implies that teleseismic waves can cause a consolidation of well aquifer and cause the increase of effective pressure (decrease of permeability and porosity), which is in accordance with the increase of co-seismic water level changes accompanied with the increase of Skempton’s coefficient B in well a, b, c, and d.”

As show in equation (5):
$$B = -\frac{3\rho g(1-2\nu_u)}{2G(1+\nu_u)} \frac{\Delta h}{\Delta \varepsilon_t}$$
 B can be influenced by shear modulus G, and the

poisson’s ratio ν_u and we have testified that: the variation of the two parameters before and after earthquake can be neglected compared with the variation of Skempton’s coefficient B. [Please see “Assumptions of shear modulus and Poisson’s ratio and the calculation of Skempton’s coefficient B” Line 116—146.](#)

- p 10 and all the discussion on permeability is confusing. Are there permeability changes (as p 10 says) or not (1 350-355)?

Answer: Yes, after read the whole paper, we really find it is confusing, especially as a new reader. So we have do an enormous modification, see those annotated (green color) portions in the mechanism analysis, [See Line: 190-447.](#) (“[Mechanism analysis](#)”)

- The authors claim there is no issues with hydraulic coupling due to large water storage. But phase lag is not the same before and after the earthquake in some wells. This may be also the sign of change in permeability. Note finally, that your tidal analysis gives only phase with 1 hour of resolution: for M2, that is a phase lag of 30°, which is enormous. Do you have an estimate of permeability and wellbore storage to discard any issue with hydraulic coupling, using directly the equation of Hsieh, WRR, 1987 ?

- To show that only B is changing, analyzing M2 may not be enough. One can try to redo the analysis with O1 tidal component, to check that phase is not changing (phase resolution is better with ~24h, the hydraulic coupling should be also better, and the same results should be found). Also the barometric efficiency should change in the same amount as B if the other coefficients are unaffected. This independent analysis would improve the discussion on the cause of the tidal

changes, by deciphering the effect of poroelasticity and hydrology in the tidal changes.

Answer: These are good suggestions, however, as explained by Hsieh *et al.* (1987), their analysis suggests that: the computed O1 phase shift is subject to large uncertainty, while the computed M2 phase shift is substantially more accurate. So we use the M2 wave to calculate the phase shift. The enormous phase shift may be attributed to the earthquake, which induced the variation of the parameters (permeability/porosity, Skempton's coefficient B) in the aquifer.

From our study we find lots of factors will influence the far-field co-seismic water level changes, such as lithology, topography and geometry of the well, and it is necessary to calculate the Transmissivity (permeability), so as to testify the mechanism. The commonly used permeability calculation [based on equations of Hsieh *et al.* (1987)] is based on several parameters: the dimensionless storage coefficient S , the radius of the screened or open portion of the well r_w , the radius of the well casing r_c . Because we lack the lithologic logs for all those wells, it is hard to confirm r_w or r_c for them. In our study we find the permeability increase in 4 wells (well f, h, i and Fuxin), only well f and Fuxin well have the records of well logs. However, there are no records of r_w and r_c in <China earthquake monitoring record series>, and it is hard to confirm r_w or r_c from the logging figures (Figure 6) for the two wells. We have to neglect the calculation of permeability in this paper, alternatively, we use the phase lag between water level and tidal strain to approximately estimate the variation of permeability. Later, we may focus on 1—2 wells, which have detailed records of borehole data, water level, and seismogram, and then we may do analysis of the permeability, together with the Skempton's coefficient B , so as to do comparison and to reveal the mechanism more deeply and clearly. **See: Line 626—643** And see part 4.4 “Well storage effects”, **Line: 409—447**

- You try to apply your model to a variety of geological settings, suggesting a universal behavior. I thought the Chinese Earthquake Administration had a much larger number of monitored wells. Do you have examples of wells not evolving, or with other changes in B than what is expected in your model? If yes, why does your model not work?

Answer: Yes the Chinese Earthquake Administration had a much larger number of monitored wells, however, as discussed in the “Selection principle”, lots of wells in the far field (the epicentral distance >1000 km) has no obvious co-seismic water level changes, and some of those wells lay near the sea, which will be affected by the ocean tides, so as indicated by the first two reviewers, we neglected those wells.

Well e is out of our expectation, **as show in Line 435—447**. “Except for well e (Table 5), it is out of our expectation. Although there is no obvious records of shales in the lithologic logs of well e, there are shales (may be a small quantity of shale) in the aquifer lithology according to the <China earthquake monitoring record series> (Table 1), and the permeability in well e may be relatively high, so it connects well with the place outside, thus there is a low probability of connecting to a place of different pressure. Phase lag increases (which indicates a decrease in permeability) accompanied with the increase of water level in well e. In our expectation, this situation should incur an increase in Skempton's coefficient B (an increase in effective pressure), which indicates the aquifer be consolidated (squeezeed). However, the effective pressure (Skempton's coefficient B) decrease in well e, this may be attributed to the fast decrease of water level after the earthquake (Figure 2). Further researches need to be done so as to detect the mechanism more clearly.”

Finally, as a 3rd reviewer, I support the request of the two first reviewers:

- the request for logs was to better characterize the aquifers. Are they porous? Fractured? Do the wells sample multiple

aquifers? What are the constraints 制约因素 (tests on cores, sonic logs) to calibrate 校准 the elastic coefficients that are needed to extract correct values of Skempton coefficient? These questions can be answered more precisely than by stacking raw lithological logs.

Answer: This is a good suggestion (especially as indicated by the reviewer, to consider about the shale in the aquifer). As indicated by this reviewer, those log analysis are very useful. Especially the aquifer with shales, which may display a (fractured) high permeability nature, and this helps us to analyze the mechanism much more deeply. See: Part “4.3 Well lithologic logs and permeability” [Line 375–408](#) (and also Abstract, Introduction)

- The request for seismograms. It seems that other earthquakes, and especially the aftershocks of Wenchuan earthquakes did not trigger any changes. How do they compare? How much less are the PGA (Peak Ground Acceleration) and PGV (Peak Ground Velocity)? How did the shaking spectra change?

Answer: Yes, the seismogram analysis is meaningful. Those wells are all in the far field (the aftershocks of Wenchuan earthquake did not trigger any obvious changes in water level), we use the seismograms mainly to do comparisons between the arrival time of surface waves and the occurrence time of co-seismic water level changes. [See “Compare with seismograms” Line: 505–552, and Table 6](#)

There are aftershocks, and the one following the M_s 8.0 main shock (Chinese time 14:27:59.5) is at 14:43:14.7, it is about 15 minutes later, so it will not cause disturbances on the main shock seismogram. What’s more the after shocks are much smaller (the magnitude of aftershocks are less than M_s 6.0) than the main shock, the energy will decrease about 900 times, when the magnitude decrease 2, so the energy of those aftershocks are much smaller, which are not large enough to induce the variation of water level. [See Line: 546–552](#)

As pointed out by the reviewer, we show the seismograms and the PGV in [Figure 8](#). “The PGV (peak ground velocity) of Fuxin (SNY station) is about 3.224 mm/s, and that of well (k) (HEF station) is about 6.891 mm/s. Although the co-seismic water level changes in Fuxin is smaller than that in well (k), since they are induced by different mechanisms (co-seismic water level ($\Delta h=0.121\text{m}$) in Fuxin is induced by increased permeability followed by a rapid redistribution of pore pressure, and co-seismic water level ($\Delta h=-0.455\text{m}$) in well (k) is induced by dilatation), the ratio of PGV should not directly related with the ratio of co-seismic water level changes in the two wells.” [See Line: 538–545](#)

To conclude, given the amount of comments from my part and from the other reviewers, I suggest the paper to be rejected, and I encourage resubmission with a major reworking of the paper.

Studies of mechanisms for water level changes induced by teleseismic waves

1 Yan Zhang¹, Li-Yun Fu¹, Fuqiong Huang², Lian-feng Zhao¹, Yuchuan Ma², Bo Zhao²,
2 and Benxun Su¹

3 1. Key Laboratory of the Earth's Deep Interior, Institute of Geology and Geophysics,
4 Chinese Academy of Sciences, No. 19, Beitucheng Western Road, Beijing 100029,
5 China. E-mail: eve_041744@163.com; lfu@mail.iggcas.ac.cn

6 2. China Earthquake Networks Center, No. 5, Sanlihenanhang Avenue, Beijing 100036,
7 China.

Abstract

8 The M_s 8.0 Wenchuan earthquake of May 12, 2008 induces large-amplitude
9 water level changes at intermediate and far fields (epicentral distance >1.5 fault
10 rupture length) in Chinese mainland. Although many hydrologic changes induced by
11 teleseismic waves have been reported, the mechanisms responsible for the changes
12 still remain unclear. We invoke Skempton's coefficient B and effective pressure in this
13 paper to explain those co-seismic water level changes documented in the intermediate
14 and far fields. The most used "enhanced permeability with a rapid redistribution of
15 pore pressure induced by removing loose particles from fractures by teleseismic
16 waves" can not be applied to explain all those coseismic water level changes in this
17 study. From our research we find some of those abrupt coseismic water level changes,
18 for which the variation of the co-seismic water level, and the effective pressure
19 preserve consistent (all increase or all decrease) are found to favor the consolidation
20 (porosity decrease) / dilatation (porosity increase) induced by the shaking of

21 teleseismic waves. Most of those wells have relatively high permeabilities attributing
22 to the shales in the aquifer lithologies. While the other part of those coseismic water
23 level changes (the variation of the co-seismic water level keeps inconsistent with the
24 variation of effective pressure), can be explained with the enhanced permeability with
25 a rapid redistribution of pore pressure, which is caused by fracture clearing or
26 overcoming the capillary entrapment in porous channels of the aquifer induced by the
27 shaking of teleseismic waves (most probably long period surface waves). Most of
28 those wells stay in basins or hollows, this kind of terrain inclines to lead to
29 heterogeneous pore pressure in close proximity.

Introduction

30 Various hydrologic responses to earthquakes have been documented (*Kayen et al.*,
31 *2004*; *Elkhoury et al.*, *2006*; *Sil and Freymueller*, *2006*; *Chadha et al.*, *2008 I*;
32 *Wang and Manga*, *2010*), many occurred at great distances from the ruptured fault
33 where static stress changes are relatively small. Hydrologic changes induced by
34 teleseismic waves have been investigated in several studies of water wells (*Roeloffs*,
35 *1998*; *Brodsky et al.*, *2003*; *Elkhoury et al.*, *2006*; *Geballe et al.*, *2011*). Earthquake
36 induced water level changes at distant locations were reported after the Denali
37 earthquake (*Brodsky et al.*, *2003*; *Kayen et al.*, *2004*; *Sil and Freymueller*, *2006*).
38 Seismic oscillations, due primarily to surface waves from distant events, occur in
39 some wells tapping highly transmissive aquifers (*Liu et al.*, *1989*; *Liu et al.*, *2006*). *Sil*
40 *and Freymueller* (*2006*) developed an empirical relationship between water level
41 changes, epicentral distances and earthquake magnitude in the far-field. *Chadha et al.*
42 (*2008 I*) find wells appear to respond to regional strain variations and transient

43 changes due to distant earthquakes. [Liu and Manga \(2009\)](#) indicate that significant
44 water level changes can be driven at great distances by moderate-amplitude dynamic
45 (time-varying) stresses.

46 Several mechanisms have been proposed to explain these co-seismic changes in
47 water level. Fracture clearing and increased permeability caused by the
48 earthquake-induced dynamic stress have been widely used to explain most
49 documented far-field water level changes ([Brodsky et al., 2003](#); [Elkhoury et al., 2006](#);
50 [Wang and Chia, 2008](#); [Wang and Manga, 2010](#)). Overcoming the capillary
51 entrapment in porous channels is hypothesized to be one of the principal pore-scale
52 mechanisms by which natural permeability is enhanced by the passage of elastic
53 waves ([Beresnev, 2011](#)). Dynamic strain induced by the passage of seismic waves,
54 most probably long period surface waves might be the cause of water level changes in
55 the far-field ([West et al., 2005](#); [Sil and Jeffrey, 2006](#); [Chadha et al., 2008 II](#)). Other
56 proposed, but also unverified mechanisms include pore pressure increases caused by a
57 mechanism ‘akin to liquefaction’ ([Roeloffs, 1998](#)), shaking-induced dilatancy ([Bower
58 and Heaton, 1978](#)), increasing pore pressure through seismically induced growth of
59 bubbles ([Linde et al., 1994](#)), and fracture of an impermeable fault ([King et al., 1999](#)).
60 In addition, [Huang \(2008\)](#) observed the co-seismic water level increase may be
61 caused by the consolidation induced by the transmission of teleseismic waves in
62 Fuxin well. Experimental measurements of [Liu and Manga \(2009\)](#) indicate that
63 permeability changes (either increases or decreases) owing to dynamic stresses are a
64 reasonable explanation. [Wang et al \(2009\)](#) find that the groundwater flow associated
65 with S and Love waves may generate shear stress large enough to break up the flocs
66 in sediment pores and to enhance the permeability of aquifers.

67 In the present study, we use the Skempton’s coefficient B , the co-seismic water

68 level and the inferred effective pressure to explain the co-seismic water level changes
69 in the intermediate and far fields based on datasets from the Wenchuan earthquake in
70 the Chinese mainland. Using a poroelastic relation between water level and solid tide
71 (Zhang *et al.*, 2009), we calculate the in-situ Skempton's coefficient B both pre and
72 post earthquake (which are two independent quasistatic processes). From the research
73 we find: Consolidation/dilatation induced by shaking of teleseismic waves, may
74 account for the mechanism of those abrupt coseismic water level changes, for which
75 variations of co-seismic water level and effective pressure preserve uniformity. Most
76 of those wells have relatively high permeabilities attributing to the shales in the
77 aquifer lithologies. While, the other part of those coseismic water level changes, for
78 which the co-seismic water level and the effective pressure change with inconformity
79 (most of those wells stay in basins or hollows), may be explained with the increased
80 permeability caused by teleseismic waves, which in turn lead to the redistribution of
81 pore pressures. Compare the occurrence time of water level changes with the arrival
82 time of surface waves in two stations, we find the co-seismic water level changes are
83 induced by the long period surface waves.

Selection Principles and Observations

84 Large numbers of stations with co-seismic water level changes induced by
85 M_s 8.0 Wenchuan earthquake have been collected in the intermediate and far fields
86 (>1.5 fault-rupture lengths). Most of those water level changes in this area can not be
87 induced by the change of the static strains, which are extremely tiny (Zhang and
88 Huang, 2011). We selected those co-seismic water level changes with distinct
89 amplitude (tiny or obscured co-seismic water level changes have been excluded). In

90 order to calculate the pre- and post- earthquake B values, water level data in stations
91 should not be long-time missing or be influenced by other factors, such as pumping or
92 other disturbances, and the data should be long enough (at least with a 10-day
93 continuous data before and after the earthquake respectively), so that we can use the
94 least-square fit to calculate B (Appendix). In addition, the oceanic tides has been
95 known to have an effect several tens of kilometers away from the seashore (Beaumont
96 and Berger, 1975). The deformation caused by ocean tide loading is difficult to
97 calculate, these tides appear with the same frequencies as the solid earth effects (Khan
98 and Scherneck, 2003), and the tides are strongly affected by the complicated
99 topography around the seashore (Walters and Goring, 2001), so we can't simply to
100 calculate the oceanic tides by theory models. Besides, there are no public software to
101 calculate the China national offshore ocean tides, so we have to delete those wells (4
102 wells: Hejiazhuang, Huanghua, Wafangdianloufang and Yongchun) which may be
103 influenced by the ocean tides seriously. Bearing those rules in mind, we find 11
104 stations (well a to well k (Figure 1)) can be chosen during the Wenchuan earthquake
105 (Table 1).

106 Detailed basic information of each well are show in Table 1 , including well
107 depth, well diameter, aquifer lithology, and geological structure. However, diameter of
108 well g, h and j can not be found. All the water level recording instruments in those
109 wells (well a to well k) are digital, they are LN-3A digital water level instrument
110 (except for Mile well it uses LN-4A digital water level instrument, and Fuxin well
111 uses the SQ digital water level instrument), with the observation accuracy $\leq 0.2\%$ F.S. ,
112 and the sampling rate of 1/min, the resolution ratio is 1mm. We use the Mapseis
113 software (Lu *et al.*, 2002) to calculate the tidal strain data (hourly data). In order to
114 keep in accordance, both the water level and the tidal strain use the hourly data when

115 calculating the Skempton's coefficient B .

Intermediate and Far Field Analysis

Assumptions of shear modulus and Poisson's ratio and the calculation of Skempton's coefficient B

116 Calculations are performed using $\rho = 1000 \text{ kg/m}^3$, $g = 9.8 \text{ m/s}^2$, and $\nu_u = 0.29$
117 according to equation (A5) (Appendix). We suppose the undrained Poisson's ratio
118 $\nu_u = 0.29$ both pre and after earthquake, and this kind of assumption is always used
119 to simplify calculation issues of rocks near the crust (Zeng, 1984). In addition, based
120 on the poroelastic theory, and limited to isotropic conditions, Theo *et al.*(2002) aim to
121 determine the elastic material constants of the solid matrix with two level of porosities.
122 As it is not possible to experimentally determine the elastic material constants of the
123 solid matrix at these levels, a theoretical approach is presented, based on experimental
124 data taken from literature. They find different porosities lead to different values of
125 elastic modulus. Their results indicate that the variation extents of Skempton's
126 coefficient B and the bulk modulus are much larger than the drained and undrained
127 poisson's ratios (variation extent of B : 6.3% ; variation extent of K : 7.96% variation
128 extent of ν_u : 0.3%). So we can approximately assume that compared to the
129 variations of Skempton's coefficient B , the change of the undrained poisson's ratio
130 can be neglected before and after the earthquake.

131 Gassmann (1951) predicted that the effective shear modulus would be
132 independent of the saturating fluid properties (the shear modulus is a constant) in the
133 undrained isotropic poroelastic media. As studied by Berryman (1999) and Berryman
134 and Wang (2001), the theory applies at very low frequencies. At high enough
135 frequencies (especially in the ultrasonic frequencies), as the numerical simulation of

136 [Berryman and Wang \(2001\)](#) shows (based on the effective medium theory, and use a
137 complete set of poroelastic constants for drained Trafalgar shale), with the increase of
138 Skempton's coefficient B , the bulk modulus changes by as much as 100% in this
139 example, whereas the shear modulus changes by less than 10%, and other rock
140 examples also show similar results ([Berryman and Wang, 2001](#)). As discussed above,
141 we can know: It is obvious that the change of shear modulus G is tiny, and even can
142 be neglected (both in the drained or undrained cases) as compared with the change of
143 Skempton's coefficient B . In this paper we suppose, shear modulus of well aquifer
144 systems will not change after affected by the seismic waves (the frequencies of
145 seismic waves are much lower than the ultrasonic frequencies, so the change of the
146 shear modulus will be neglectable compared to the change in B value).

147 We apply the B -calculation method ([Appendix](#)) to those well-picked stations.
148 The pre-and post-earthquake B values are respectively obtained from May 1, 2008 to
149 May 11, 2008, and from May 13, 2008 to May 24, 2008 ([Figure 2](#)).

Undrained Skempton's coefficient B as a function of effective pressure

150 When the aquifer be consolidated, the effective pressure (effective pressure =
151 confining pressure - pore pressure) will increase, while a dilation is in accordance to
152 the decrease of effective pressure. [Blocher et al. \(2009\)](#) measured the relationship
153 between Skempton's coefficient B and effective pressure based on the laboratory
154 experiment. The in-situ aquifer of those wells (well a~k) we studied are under
155 lithostatic pressures for a long time and also be affected by the transmission of
156 seismic waves for countless times, the situation is much similar to those well bedrocks
157 be applied on repeated pressure cycles. So the situation will be much similar to the
158 last several ramps (apply more than once pressure cycles on the rock) rather than the

159 first ramp (apply the first pressure cycle on the rock, during which a possible
160 dissolution of gas in the fluid of an incompletely saturated sample happened) in the
161 experiment of Blocher *et al.* (2009), and the isotropic Skempton's coefficient B will
162 increase/decrease with the increase/decrease of effective pressure (when the effective
163 pressure is less than ~ 4 Mpa), while B will decrease with the increase of effective
164 pressure (when the effective pressure is larger than ~ 4 Mpa). Although these results
165 obtained from sandstone, because of the lack of the laboratory experiment study of
166 those specific rocks, we assume the results can be applied to the bedrock of all those
167 wells studied in this paper.

168 In order to compare with the experiment results, we have to estimate the
169 effective pressure of each well. Pore pressure response to gravitational loading is
170 similar to tectonic loading and can also be treated as a poroelastic problem (Green and
171 Wang, 1986). Depths of those wells analyzed in this paper are all less than 1km
172 (Table 1). W-1 well lies in Yanchang basin of Gansu province, Yanchang basin is a
173 deep basin with Paleozoic sediments (Wu *et al.*, 2010). The "pressure - depth"
174 relation of well W-1 (Figure 3a) is similar to other wells in the Chinese mainland. So
175 we assume those results could be applied to these wells we studied (well a \sim k) since
176 we lack the "pressure-depth" predictions of these wells. We calculate the effective
177 pressure of W-1 well (effective pressure approximately equals to lithostatic pressure
178 minus pore fluid pressure) (Figure 3b), and estimate the range of the effective
179 pressure of these wells we studied according to the well-depth (Table 1).

180 We calculated the change of pore pressure in each well ($\Delta P_p = \rho g \Delta h$), together
181 with the range of the effective pressure, the variation trend of Skempton's coefficient

182 B , and the B -effective pressure relation obtained by the experiment of Blocher *et al.*
183 (2009), we can infer the variation quantity of the effective pressure in each well
184 (Table 2, Table 3). When the range of the effective pressure lies in 0-3 Mpa (most of
185 the wells), the increase/decrease of B accompanied with the increase/decrease of
186 effective pressure. When the range of effective pressure >5 Mpa, the
187 increase/decrease of B accompanied with the decrease/increase of effective pressure
188 Blocher *et al.* (2009), only the effective pressure of Jurong well (well f) lies in this
189 range (Table 2).

Mechanism analysis

190 Till now, fracture clearing (unclogging) and increased permeability has been
191 used to explain most of those coseismic water level changes in the far field (Brodsky
192 *et al.*, 2003; Wang, 2007; Wang and Manga, 2010). Since pore-pressure heterogeneity
193 may be the norm in the field, an enhancement of permeability among sites of different
194 pore pressure may cause pore pressure to spread (Roeloffs, 1998; Brodsky *et al.*, 2003;
195 Wang, 2007; Wang and Manga, 2010). Analysis of well response to tidal forcing
196 before and after an earthquake has provided strong evidence that earthquakes can
197 enhance permeability (Elkhoury *et al.*, 2006). In this study, we calculate the change of
198 Skempton's coefficient B and effective pressure, however, we can not use the
199 enhanced permeability theory to explain all those coseismic water level changes. And
200 we find the other part of water level changes may favor the consolidation or dilatation
201 induced by teleseismic waves (about 58.3% of all those wells analyzed in this paper
202 favor this explanation).

203 Permeability will increase/decrease, which is mostly related to the
204 increase/decrease of porosity (Xue, 1986). As explained by rock mechanics the same
205 porosity always corresponding to the same effective pressure (Terzaghi, 1925;

206 Magara, 1978). From that we can know porosity and permeability are all directly
207 connected with effective pressure, and they will decrease with the increase of the
208 effective pressure (Blocher *et al.*, 2009).

209 We can summarize the variation of effective pressure ($P_{eff} = P_c - P_p$) in two ways:
210 (P_c confining pressure, P_p pore pressure, and P_{eff} effective pressure)

211 A) Pore pressure P_p keeps constant, the change of effective pressure P_{eff}
212 induced by the change of confining pressure P_c .

213 There are two states (Table 4): (a1) Confining pressure increases (pore pressure
214 not change), then effective pressure increases, the porosity will decrease (a process of
215 consolidation or squeeze), and water level / pore pressure will increase; (a2)
216 Confining pressure decreases (pore pressure not change), then effective pressure
217 decreases, the porosity will increase (a process of dilatation), and water level / pore
218 pressure will decrease. (a1), (a2) can be summarized as a mechanism of water level
219 change induced by consolidation or dilatation, and water level changes in accordance
220 with the change of effective pressure (all increase or all decrease) in this case.

221 B) Confining pressure P_c keeps constant, the change of effective pressure P_{eff}
222 induced by the change of pore pressure P_p .

223 There are two states (Table 4): (b1) Pore pressure/ water level decreases
224 (Confining pressure not change), then effective pressure increases, the porosity will
225 decrease (a process of water level flows out of the well to a place with a relatively
226 lower pore pressure); (b2) Pore pressure/ water level increases (Confining pressure

227 not change), then effective pressure decreases, the porosity will increase (a process of
228 water level flows into the well from a place with a relatively higher pore pressure).
229 (b1), (b2) can be summarized as a mechanism of water level change induced by
230 increased permeability with a rapid redistribution of pore pressure (this is the most
231 used mechanism for far-field coseismic water level changes), and water level changes
232 opposite to the change of effective pressure in this case.

233 As show in below (part 4.1 and part 4,2), we use two mechanisms to explain
234 those coseismic water level changes.

Coseismic water level change induced by increased permeability followed by a rapid redistribution of pore pressure

235 The effective pressure range of well h, and i is 0 ~ 3 MPa (Table 2). According
236 to the laboratory experiment of Blocher *et al* (2009), the increase of effective pressure
237 accompanied with the increase of Skempton's coefficient B in this range. Water levels
238 (pore pressure) decrease accompanied with the increase of effective pressures in well
239 h, and i (Table 2). Since pore-pressure heterogeneity may be the norm in the field, an
240 enhancement of permeability among sites of different pore pressure may cause pore
241 pressure to spread (Roeloffs, 1998; Brodsky *et al.*, 2003; Wang, 2007; Wang and
242 Manga, 2010). Pore-pressure of the two wells may be higher than the close proximity
243 before the earthquake, an enhancement of permeability incurred by (for example)
244 overcoming the capillary entrapment in porous channels induced by the passage of
245 elastic waves will decrease the pore-pressure in wells (the pore-pressure will shift to
246 other places), and water level will decrease. Then the effective pressure will increase
247 accompanied with the decrease of pore-pressure (water level), so the Skempton's

248 coefficient B increases (which indicates the stiff rock matrix could with a higher
249 coupling to the fluid) in well h and i (Table 2).

250 The depth of well f (889.18 m) is larger than other wells, and the effective
251 pressure range of this depth is 8 ~ 10 MPa (Table 2). According to the laboratory
252 experiment of Blocher *et al* (2009), the decrease of effective pressure accompanied
253 with the increase of Skempton's coefficient B in this range. Water level increases with
254 the decrease of effective pressure (increase of Skempton's coefficient B) in well f, this
255 should be explained with the increased permeability. Pore-pressure of well f may be
256 lower than the close places before the earthquake, an enhancement of permeability
257 will increase the pore-pressure in this well (the pore-pressure (water level) may shift
258 from other places), and water level (pore pressure) will increase. Then the effective
259 pressure will decrease accompanied with the increase of pore-pressure (water level),
260 supposing the confining pressure not change. As explained by Blocher *et al* (2009),
261 with the increase of effective pressure (reaches larger than 5 Mpa), the decrease of
262 the Skempton's coefficient results from the change of the pore-geometry, which leads
263 to a higher bulk modulus of the sample. Pore throats and microcracks were closed,
264 and the stiff rock matrix could with a lower coupling to the fluid, so the Skempton's
265 coefficient B decreases. And this is an reversible process (after they raised the
266 confining pressure from 5 to 50 Mpa, they lowered the confining pressure form 50 to
267 5 Mpa, and also obtained the similar results), so when the effective pressure decreases
268 (not lower than 5 Mpa), the closed pore throats and microcracks will be opened and
269 turn larger under the effect of pore pressure, the stiff rock matrix could with a higher
270 coupling to the fluid in well f, leading to the increase of Skempton's coefficient B .

271 The local geological structure of each well is important (Table 1). We find that
272 most of those wells in which the coseismic water level changes can be explained with

273 “ the enhanced permeability with a rapid redistribution of pore pressure” stay in
274 basins or in hollows (well f, h, i and Fuxin). The terrains of those wells incline to lead
275 to heterogeneous pore pressures in close proximities (possibly attributed to different
276 altitudes).

Coseismic water level change induced by consolidation or dilatation

Coseismic water level change induced by dilatation

277 For well g, j and k, the effective pressure range is 0 ~ 3 MPa, effective pressure
278 will increase/decrease accompanied with the increase/decrease of Skempton’s
279 coefficient B during this range (Blocher *et al.*, 2009). Water levels (pore pressures) of
280 well g, j and k decrease, accompanied with the decrease of effective pressures [and
281 decrease of Skempton’s coefficient B (which indicates the stiff rock matrix could with
282 a lower coupling to the fluid)], which can not be explained with the increased
283 permeability followed by the rapid pore pressure redistribution between the well and
284 the places near the well. Whereas, this could be explained with the state (a2)
285 Confining pressure decreases (pore pressure not change), then effective pressure
286 decreases, the porosity will increase (a process of dilatation), and water level / pore
287 pressure will decrease.

288 The spreading of teleseismic waves may cause dilatation of the aquifer medium,
289 which can broaden the porosities (the permeability will increase) and give birth to
290 new fractures, and the effective pressure will reduce (in wells: g, j and k) leading to
291 the decrease of Skempton’s coefficient B . This explanation is similar to the
292 mechanism of shaking-induced dilatancy (Bower and Heaton, 1978).

Coseismic water level change induced by consolidation

293 For well a, b, c, and d, the effective pressure range is approximately 0 ~ 3 MPa,
294 effective pressure will increase/decrease accompanied with the increase/decrease of
295 Skempton's coefficient B (Blocher *et al.*, 2009). Water level (pore pressure) of well a,
296 b, c, and d increase, accompanied with the increase of effective pressure [and increase
297 of Skempton's coefficient B (which indicates the stiff rock matrix could with a higher
298 coupling to the fluid)], which also can not be explained with the increased
299 permeability followed by the rapid pore pressure redistribution between the well and
300 the place near the well. Whereas, this could be explained with the state (a1)
301 "Confining pressure increases (pore pressure not change), then effective pressure
302 increases, the porosity will decrease (a process of consolidation or squeeze), and
303 water level / pore pressure will increase". This mechanism is very similar to the
304 explanation of the laboratory experiment of Liu and Manga (2009). From their
305 laboratory experiment, they find that: in general, permeability/porosity decreases after
306 shaking. They measured the evolution of permeability in fractured sandstone in
307 response to repeated shaking under undrained conditions, and set the frequency and
308 amplitude of the imposed shaking to be representative of those that cause distant
309 hydrological responses. As they explained: Dynamic strains cause time varying fluid
310 flow that can redistribute particles within fractures or porespace, and can allow
311 particles to move away from regions where they hold pore spaces open, and are
312 expected to accumulate and get trapped at the narrowest constrictions along flow
313 paths, and hence allow a consolidation (contraction) of the sample, which can lead to
314 a higher coupling between the stiff rock matrix and the fluid. Their result just supports
315 our mechanism analysis. It implies that teleseismic waves can cause a consolidation
316 of well aquifer and cause the increase of effective pressure (decrease of permeability
317 and porosity), which is in accordance with the increase of co-seismic water levels

318 accompanied with the increase of Skempton's coefficient B in well a, b, c, and d.

Examples support far field water level increases induced by consolidation

319 We analyze the mechanism of the coseismic water level changes induced by
320 consolidation incurred by teleseismic waves in above. However, water level increases
321 induced by consolidation in the far field is not the mainstream view. It is necessary to
322 give some examples which can support far field water level increases induced by
323 consolidation.

324 Huang (2008) find that: the water level increase in Fuxin well (1409.98 km away
325 from Wenchuan, the well depth is 60.74 m, stiff Granite with a little basalt is the
326 bedrock and we assume the shear modulus = 60 Gpa) is induced by the increase of
327 volume strain (consolidation) (Figure 4a). In the Chinese mainland, Fuxin is the only
328 well in which there are observations of volume strain and water level in a specific
329 aquifer medium, and both of them show obvious co-seismic responses to Wenchuan
330 earthquake. There are clear and obvious effects of tidal strain and atmospheric
331 pressure in the water level and volume strain, which indicates Fuxin is a terrific
332 artesian well. This well has not be chosen in the above analysis because there is an
333 abrupt large-amplitude increase in the water level, which starts from 11 p.m. May 22,
334 2008 (we can not find any interference of this abrupt increase according to the daily
335 records of Fuxin station), and we can just use a shorter time period to calculate the
336 post-earthquake B value, which may cause a little impact on the precise of B . The
337 calculation is performed based on the M_2 wave distilled from the water level and
338 the tidal strain (pre-earthquake: from May 1, 2008 to May 11, 2008, post-earthquake:
339 from May 13, 2008 to May 22, 2008 (Figure 4b)). (The large-step abrupt water level
340 increase starts from 09 p.m. May 22, 2008 (Figure 4c), which may cause large impact

341 on the detrend process and influence the calculation result, so we discard these data).
342 From [Figure 4a](#), we can see the coseismic water level increase is induced by the
343 change of the volume strain, which indicates the well aquifer has been consolidated.
344 The depth of Fuxin well is 60.74 m, and we can assume the range of the effective
345 pressure is 0~3Mpa ([Table 2](#)), from the change of the pre- and post- earthquake B
346 ([Figure 4b](#)), we may infer the consolidation may be very extreme, accompanied with
347 the coseismic water level increase it could cause an extra pressure, which overcomes
348 the capillary entrapment in porous channels of the aquifer or incurs a fracture
349 clearing and bring in the increase of the permeability, then water flow in from other
350 places with a higher pressure, which lead to the decrease of the Skempton's
351 coefficient B with the decrease of the effective pressure, and the water level increases
352 more gradually (corresponding to the state (b2)). Finally with the further enhancement
353 of the permeability (increase of the porosity), a permanent deformation could be
354 induced, so there is an abrupt increase in the water level in 22 May, and remain in a
355 relatively high level for several months([Figure 4c](#)). From the picture we can see it
356 may be in a drained condition after the abrupt large-amplitude water level increase,
357 because the water level fluctuates irregularly.

358 So we argue that water level increase induced by the consolidation incurred by
359 transmission of teleseismic waves is reasonable, and in a specific geology condition,
360 a consolidation with large enough energy may also lead to an enhanced permeability
361 by fracture clearing or by overcoming the capillary entrapment in porous channels.

Conclusion of coseismic water level changes induced by consolidation or dilatation

362 Water level increases/decreases accompanied with the increase/decrease of
363 effective pressure (and the increase/decrease of Skempton's coefficient B) in well a, b,

364 c, d, g, j, and k (the effective pressure range is approximately 0 ~ 3 MPa) (Table 3).
365 To our understanding, suppose the pressure not exceed a limitation (a permanent
366 deformation not happened), when the aquifer be consolidated/ dilatated, the mean
367 fracture width (the porosity and permeability) may decrease/increase with the
368 increase/decrease of the effective pressure, then the stiff rock matrix that supports the
369 load could with a higher/lower coupling to the fluid (Nur and Byerlee, 1971), and the
370 value of B will increase/decrease. Hence, shaking induced by the transmission of
371 teleseismic waves may cause consolidation/dilatation of the aquifer, and lead to the
372 increase/decrease of the water level. Figure 5 shows the relation between the change
373 of Skempton's coefficient B and the change of effective pressure in well a, b, c, d, g, j,
374 and k . Approximately, it displays a linear relation.

Well lithologic logs and permeability

375 As indicated by Wang *et al.* (2009) High transmissivity promotes uniform pore
376 pressure, thus there is a low probability of connecting to a reservoir of different
377 pressure. On the other hand, poor transmissivity can support heterogeneous pore
378 pressure in close proximity, thus there is a high probability of connecting to a
379 reservoir of different pressure. We show the well lithologic logs (borehole columnar
380 diagrams) in Figure 6. According to <China earthquake monitoring record series>
381 [which is written by different Subordinate units (earthquake administration of each
382 provinces and different institutions) of China Earthquake Administration, and
383 published in Beijing in different years by Seismological Press (in Chinese)], we can
384 only get the lithologic logs of well (a), (c), (d), (e), (f), (k) and Fuxin (Figure 6), the
385 pictures are designed already, some lithologic logs are explained in detail and some
386 are in shot. Shales display in Lithologic logs of well (c), (d) , (e) [Although there is no

387 obvious records of shales in the log of well e, according to the <China earthquake
388 monitoring record series> there are shales (may be a small quantity of shale) in the
389 matrix rock of well (e) (Table 1)] and (k), so the permeability of well (c), (d), (e) and
390 (k) may be relatively larger, and they are well connected with the close places outside
391 the well, so there are little pressure differences between those wells and the places
392 around them, and water level tend not to flow into or out of those wells even if the
393 porosity/permeability increased by the shaking of telesismic waves. On the other hand,
394 there are no shales in the logs of well (a), (f) and Fuxin, and the permeability may be
395 relatively smaller, thus there is a high probability of connecting to a reservoir of
396 different pressure, and water level tend to flow into or out of those wells after the
397 porosity/permeability increased by the shaking of telesismic waves. Except for well
398 (a), those analyses of lithologic logs are in accordance with our above mechanism
399 analyses [well (c), (d), (e) and (k) favor the consolidation/ dilatation mechanism,
400 while well (f) and Fuxin are induced by the increased permeability followed by a
401 rapid redistribution of pore pressure]. Other factors may also influence the
402 permeability, such as the geometry of the well. However, we can not make further
403 study about that, since we lack the in situ tests and the detailed lithologic logs of other
404 wells.

405 Because we use the approximately median value of shear modulus G (Table 1) to
406 calculate the Skempton's coefficient B , it is hard to estimate the permeable extent of
407 the aquifer from the absolute value of B . Whereas, the variation tendency of B before
408 and after the earthquake is definite.

Wellbore storage effects

409 Tidal phase lags are caused by wellbore storage. "Wellbore storage" is the term
410 used to describe a lag of piezometer water level behind aquifer pressure resulting

411 from the need for water to flow into the borehole in order to equilibrate water level
412 with aquifer pressure. Wellbore storage effects are a function of the transmissivity
413 between the well and aquifer, in addition to the geometry of the well (Cooper *et al.*,
414 1965; Liu *et al.*, 1989; Kano and Yanagidani, 2006). Wellbore storage effects increase
415 (phase lags increase) as the transmissivity (and permeability) of the formation
416 decreases (Roeloffs, 1996; Doan *et al.*, 2006).

417 Most of those wells can record clear tidal strain and atmospheric pressure, and
418 according to the <China earthquake monitoring record series> they are well confined.
419 From Table 1 we can see the phase difference of water level and tidal strain of most
420 wells are 0, which mean good correlations between the water levels and the tidal
421 strains, and those wells are well confined and under the undrained condition. Hsieh *et*
422 *al.* (1987) indicates that: the computed O1 phase shift is subject to large uncertainty,
423 while the computed M2 phase shift is substantially more accurate. So we use the M2
424 wave to calculated the phase shift. Because we use the hourly data, we can not
425 identify the phase difference when it is less than 1 hour, and we neglected the
426 wellbore storage effects in those wells. Before and after the earthquake, if phase lags
427 remain the same, it indicates the permeability of the well aquifer keeps the same or
428 changes not much (the phase difference may be lees than 1 hour). Phase lags ≥ 1
429 hour in well b, c, e, and Fuxin, and most of them are small, except well b, which may
430 be semi-confined. Thus, the validity of the calculated *B* values in well b may be a
431 little questionable. The phase lag of Fuxin well decreases after the earthquake (L1=2
432 hours, L2=1 hour), which indicates the permeability increases after the shakig of the
433 earthquake, this is in accordance with the mechanism analysis of the co-seismic water
434 level increase in Fuxin well.

435 Except for well e (Table 5), it is out of our expectation. Although there is no

436 obvious records of shales in the lithologic logs of well e, there are shales (may be a
437 small quantity of shale) in the aquifer lithology according to the <China earthquake
438 monitoring record series> (Table 1), and the permeability in well e may be relatively
439 high, so it connects well with the place outside, thus there is a low probability of
440 connecting to a place of different pressure. Phase lag increases (which indicates a
441 decrease in permeability) accompanied with the increase of water level in well e. In
442 our expectation, this situation should incur an increase in Skempton's coefficient B
443 (an increase in effective pressure), which indicates the aquifer be consolidated
444 (squeezed). However, the effective pressure (Skempton's coefficient B) decrease in
445 well e, this may be attributed to the fast decrease of water level after the earthquake
446 (Figure 2). Further researches need to be done so as to detect the mechanism more
447 clearly.

Discussion

The variation of porosity

448 Figure 3c shows, in general, the porosity decreases with the increase of depth,
449 however, when reach 3000m the effective pressure turns much larger (approximately
450 equals to 35 Mpa) than that in the depth of those wells (well a ~ k), the porosity still
451 persists relatively large, and changes with different depth. From Table 3 we can see,
452 the variations of effective pressure in well a, b, c, d, g, j and k are less than 0.01Mpa,
453 and from Figure 3b we know, variation of 0.01Mpa in effective pressure
454 approximately equals to variation of 1 meter in depth, as Figure 3c shows, the
455 variation of porosity is tiny during variation of 1 meter in depth. So this variation
456 extent of effective pressure is hard to induce permanent deformation of porosity.
457 However, in reality, the change of porosity may also connected with the formation

458 and the state of the rock matrix.

459 Furthermore, phase lags of well a, b, c, d, g, j and k keep constant before and
460 after the earthquake (change less than 1 hour) (Table 1), so we can infer, the porosity
461 (permeability) changes little after the earthquake. Because the phase lags
462 increase/decrease (wellbore storage effects increase/decrease) as the permeability
463 (porosity) of the formation decreases/increases (Roeloffs, 1996; Doan *et al.*, 2006).

464 So we can infer, the porosity of well a, b, c, d, g, j and k can persist despite being
465 reduced/enlarged due to the consolidation/dilatation induced by the passage of
466 teleseismic waves of M_s 8.0 Wenchuan earthquake.

Uncertainty of B coefficient

467 In order to study the uncertainty of B coefficient (error related to the
468 determination of B coefficient), we use Jurong well to show the variation of B during
469 a relatively long – time span (50 days before and after the Wenchuan earthquake)
470 (Figure 7). Skempton's coefficient B will change with the change of time. Because we
471 use the least square fit to calculate B , the value may be a little different when we use
472 different length of data, but the change tendency (increase or decrease of B) before
473 and after the earthquake will be constant. Furthermore, we can see the B value of
474 Jurong well recover to its initial value after about 30 days (Figure 7).

475 So, compared with the uncertainty in B value, variation of B due to the
476 earthquake is significant. The continuous of B will be influenced by lots of factors,
477 such as power off, aftershocks, and so on, so B -value series at large time scale is not
478 easy to obtain for each well.

Recovery of Water level

479 The recovery time of the water level is obscure, because most of those water

480 levels will not recover to the pre-earthquake heights during a relatively short time
481 span. So we should use much longer data to analyze it, and should discard all those
482 influences: such as aftershocks, atmospheric pressure (not all those wells have the
483 records of atmospheric pressure) tidal strain, pumping, power off, thounder and so on,
484 which needs lots of work, and we may study about it in future. In addition, we haven't
485 find any relation between water level changes and epicentral distances in those wells
486 studied in this paper, it is possible to investigate much more wells later, to study about
487 the relations.

The variation value of effective pressure

488 We calculat the change of pore pressure ($\Delta p_p = \rho g \Delta h$), and we can use the critical
489 state to help us to analyze the variation value of effective pressure in each well.

490 When the aquifer be consolidated/dilated, in the critical state, the pore pressure
491 keeps constant, the confinging pressure increases/decreases, then the effective
492 pressure increases/decreases, and at last transfers into the increase/decrease of pore
493 pressure (water level), and the system comes into an equilibrium state. So the change
494 of pore pressure can be attributed to the change of the effective pressure.

495 When the permeability increases, in the critical state, the confining pressure
496 keeps constant, the pore pressure (water level) increases (the well in a relatively low
497 pressure region before the earthquake) /decreases (the well in a relatively high
498 pressure region before the earthquake), then the effective pressure decreases/increases,
499 so the change of the effective pressure can be attributed to the change of pore
500 pressure.

501 However , the variation value of the effective pressure in each well may be

502 different from the value we calculate, because the critical state is an assumption ideal
503 state, and the transfer of stress may also relate with the formation and state of the
504 aquifer.

Compare with seismograms

505 There are 48 national stations recording the seismograms (event waveforms) in
506 the Chinese mainland (we can not obtain some of the regional seismograms because
507 of the authority limitation), however most of those stations are not in the same place
508 with stations which have the records of water level changes. Those stations (well a to
509 k) analyzed in our paper do not record seismograms. After comparison, generally we
510 may use the seismograms of 4 national stations to analyze the corresponding water
511 level observations, which are near those national stations (the distances between the
512 water level wells and the national seismogram stations are approximately less than
513 100km). However, there are deficiencies in the seismograms of station TIY and LZH
514 (TIY station is corresponding to well e (there are about 40.903 km between them);
515 LZH station is corresponding to well g (there are about 19.82 km between them)), so
516 we desgarded them. Finally, two seismograms can be used: the seismogram of SNY
517 (Shengyang) station is used to analyze Fuxin well (there are about 102.81 km between
518 them), and HEF (Hefei) station is corresponding to well k (there are about 91.57 km
519 between them). In addition, the geology conditions are very similar (the main matrix
520 rocks of Fuxin well and Shengyang station are both granite; Well k is in Chuhe river
521 major dislocation and Hefei-Dongguan fracture intersection).

522 There are only hourly water level data in Fuxin well (minute data observation

523 strats from 2009), so we can not use that to do precise comparison (in minute) with
524 the seismogram. In general, we can only use well k to do the comparison between the
525 timing of step in water level change and the arrival time of seismic waves. From the
526 occurrence time of water level changes and the arrival time of surface waves of well k
527 (Table 6), we find the co-seismic water level changes are attributed to the passage of
528 surface waves. From that, we may infer: in other wells the co-seismic water level
529 changes are attributed to the dynamic strain induced by the passage of teleseismic
530 waves, most probably surface waves, which have relatively larger amplitude of
531 oscillation, corresponding to relatively larger energy. The similar conclusion has been
532 proposed by Sil and Jeffrey (2006), West *et al.* (2005), and Chadha *et al.* (2008 II).
533 More precise estimation of the timing of the step could not be made because of the
534 low temporal resolution of the water level data. Obviously, there are geographic
535 position differences between the observation of seismograms and water levels, and
536 there are also some errors on the manual amplitude readings, both of which could
537 cause some influence on the analysis.

538 The PGV (peak ground velocity) of Fuxin (SNY station) is about 3.224 mm/s,
539 and that of well (k) (HEF station) is about 6.891 mm/s. Although the co-seismic water
540 level changes in Fuxin is smaller than that in well (k), since they are induced by
541 different mechanisms (co-seismic water level ($\Delta h=0.121\text{m}$) in Fuxin is induced by
542 increased permeability followed by a rapid redistribution of pore pressure, and
543 co-seismic water level ($\Delta h=-0.455\text{m}$) in well (k) is induced by dilatation), the ratio of
544 PGV should not directly related with the ratio of co-seismic water level changes in the

545 two wells.

546 There are aftershocks, and the one following the M_s 8.0 main shock (Chinese
547 time 14:27:59.5) is at 14:43:14.7 , it is about 15 minutes later, so it will not cause
548 disturbances on the main shock seismogram. What's more the after shocks are much
549 smaller (the magnitude of aftershocks are less than M_s 6.0) than the main shock, the
550 energy will decrease about 900 times, when the magnitude decrease 2, so the energy
551 of those aftershocks are much smaller, which are not large enough to induce the
552 variation of water level.

Conclusion

553 Together with the variation of Skempton's coefficient B , the change of pore
554 pressure and the inferred variation of effective pressure in each well, we can infer the
555 mechanism of the co-seismic water level changes. From the study we can conclude:
556 consolidation/dilatation induced by shaking of teleseismic waves, may account for the
557 mechanisms of those coseismic water level changes, for which the variation tendency
558 of the co-seismic water level, and the effective pressure keep the same (all increase or
559 all decrease), and most of those wells have relatively high permeabilities attributed to
560 the shales in the matrix rocks (based on the obtained 7 well logs in this study). While,
561 fracture clearing and increased permeability with a rapid redistribution of pore
562 pressure may be used to explain the other part of those coseismic water level changes,
563 for which the co-seismic water level, and the effective pressure change with
564 in conformity. Most of those wells stay in basins or hollows (well f, h, i and Fuxin),
565 this kind of terrain inclines to lead to heterogeneous pore pressure in close proximity.
566 Compared with the seismograms, the co-seismic water level changes are attributed to

567 the dynamic strain induced by the passage of seismic waves, most probably long
568 period surface waves. Our analysis is not conflict with any of those existing theories.
569 Although those water level changes happened in the intermediate and far fields, most
570 of those water levels present abrupt and obvious co-seismic changes owing to the
571 huge energy of M_s 8.0 Wenchuan earthquake.

572 Experiments of Liu and Manga (2009) apply time varying axial stresses
573 (confining pressure changes) whereas Elkhoury *et al.* (2011) applied time varying
574 fluid pressure differences (pore pressure changes) across their samples. Our study
575 complement the experiments of both of them, we discusse the change of effective
576 pressure ($P_{eff} = P_c - P_p$) in two ways: A) Pore pressure P_p keeps constant, the change
577 of effective pressure induced by the change of confining pressure P_c . B) Confining
578 pressure P_c keeps constant, the change of effective pressure induced by the change of
579 pore pressure P_p .

580 From the analysis of Fuxin well, we can see consolidation also can be incurred
581 by teleseismic waves. As discussed by Liu and Manga (2009): Dynamic strains cause
582 time varying fluid flow that can redistribute particles within fractures or porespace,
583 and can allow particles to move away from regions where they hold pore spaces open,
584 and are expected to accumulate and get trapped at the narrowest constrictions along
585 flow paths, and hence allow a consolidation (contraction) of the sample. The
586 mechanism analyzed in this paper are similar to the experiment results of Liu and
587 Manga (2009), and our in-situ analysis may complement the limitation of the initial
588 condition of their laboratory experiment.

589 Matrix rocks with shales always correspond to relatively high permeabilities, and
590 those wells are well connected with the other places outside the well, so there may be

591 little pressure differences between those wells and the places around them, and water
592 levels tend not to flow into or out of those wells even if the porosity/permeability
593 increased by the shaking of teleseismic waves. From our study we find most of the
594 coseismic water level changes in those wells can be attributed to the change of
595 confining pressures. Consolidation/dilatation induced by shaking of teleseismic waves,
596 may account for the mechanism. According to the geological structures, all of those
597 wells stay in faults or in fracture intersections. Meanwhile, we find that the wells in
598 which the coseismic water level changes can be explained with “the enhanced
599 permeability with a rapid redistribution of pore pressure”, stay in basins or in hollows
600 (well f, h, i and Fuxin). The terrains of those wells incline to lead to heterogeneous
601 pore pressures in close proximities (possibly attributed to different altitudes).

602 In reality, some well aquifers are not porous and may be fractured, especially
603 those wells with shales in the matrix rocks, may display substantial anisotropy or a
604 fractured property rather than a porous property, however, we suspect that the
605 isotropic and homogeneous poroelastic theory we used here is the best available
606 approximation. The Skempton coefficients are very small for many wells, which may
607 be attributed to the value of the shear modulus G [see Zhang and Huang (2011), since
608 we lack the in-situ G values, we investigate the geology of each well and referred to
609 the <rock mass mechanism> (Liu and Tang, 1998), using the dynamic elastic modulus
610 and dynamic Poisson’s ratios to estimate the ranges of the dynamic shear modulus of
611 those matrix rocks (according to the formula $G = \frac{E}{2(1+\sigma)}$), and to choose the
612 approximate mean values (Table 1)]. The shear modulus G and the undrained
613 Poisson’s ratio ν_u would change slightly after the shaking of seismic waves, and the
614 discussed “undrained” condition can hardly last for a long time, as long as the fluid

615 flow exists, the undrained condition will disrupt and be replaced by the drained
616 condition soon. We assume the results get from sandstone (Figure 3) can be applied to
617 all those bedrocks in those wells, however this is not very precise. As described by
618 Wang (1993) nonlinear compaction effects can be significant and they are not
619 incorporated in the linear theory presented here. Because the well aquifers are under
620 lithostatic pressures for a long time and withstand large numbers of seismic shaking,
621 the irreversible deformations and the nonlinear effects have been minimized (In the
622 laboratory experiment, in order to reduce the irreversible deformation and to minimize
623 the nonlinear effects, repeated pressure cycles are always applied on rock samples as
624 preconditions (Blocher *et al.*, 2009)). Discard all those ideal assumptions, things may
625 be different.

626 From our study we find lots of factors will influence the far-field co-seismic
627 water level changes, such as lithology, topography and geometry of the well. Later it
628 is necessary to calculate the transmissivity (permeability), so as to testify the
629 mechanisms. The commonly used permeability calculation [based on equations of
630 Hsieh *et al.* (1987)] is based on several parameters: the dimensionless storage
631 coefficient S , the radius of the screened or open portion of the well r_w , the radius of
632 the well casing r_c . Because we lack the logs for all those wells, it is hard to confirm
633 r_w or r_c for all of them. In our study we find the permeability increase in 4 wells
634 (well f, h, i and Fuxin), only well f and Fuxin well have the records of lithologic logs.
635 However, there are no direct records of r_w and r_c in <China earthquake monitoring
636 record series>, and it is hard to confirm r_w or r_c from the lithologic logs (Figure 6)
637 for the two wells. We have to give up the calculation of permeability in this paper,
638 alternatively, we use the phase lag between water level and tidal strain to

639 approximately estimate the variation of permeability before and after the earthquake.
640 Later, we may focus on 1—2 wells, which have detailed records of borehole datas,
641 water level, and seismogram, and then we may do analysis of the permeability,
642 together with the Skempton's coefficient B , so as to do comparison and to reveal the
643 mechanisms more deeply and clearly.

Data and Resources

644 Data used in this paper were collected using a classified network (Groundwater
645 Monitoring Network, GMN) of the China Earthquake Networks Center and cannot be
646 released to the public. We use the Mapseis software (Lu *et al.*, 2002) to calculate the
647 tidal strain data.

Acknowledgements. We thank professor Emily Brodsky, and postdoctor
Jinlai Hao for discussion and help on this paper. This research was supported by the
Natural Science Foundation of China (Grant No. 40925013 and 41040036). We
gratefully appreciate the valuable suggestions proposed by the anonymous reviewers.

Reference

- 648 Brodsky, E., E. Roeloffs, D. Woodcock, I. Gall, and M. Manga (2003). A mechanism
649 for sustained groundwater pressure changes induced by distant earthquake, *J.*
650 *Geophys. Res.* **108(B8)**, 2390.
- 651 Beresnev, I., W. Gaul, and R. D. Vigil (2011). Direct pore-level observation of
652 permeability increase in two-phase flow by shaking, *Geophys. Res. Lett.* **38**,
653 L20302.
- 654 Blocher G., G. Zimmermann and H. Milsch (2009). Impact of poroelastic response of
655 sandstone on geothermal power production, *Pure appl. geophys.* **166**,

656 1107–1123.

657 Bower, D. R., and K. C. Heaton (1978). Response of an aquifer near Ottawa to tidal
658 forcing and the Alaskan earthquake of 1964, *Can. J. Earth Sci.* **15**, 331–340.

659 Berryman, J. G., and H. F. Wang (2001). Dispersion in poroelastic systems, *Phys. Rev.*
660 *E* **64**, 011303.

661 Berryman, J. G. (1999). Origin of Gassmann’s equations, *Geophysics* **64**, 1627–1629.

662 Beaumont, C. and J. Berger (1975). An analysis of tidal strain observations from the
663 United States of America: I. The laterally homogeneous tide, *Bull. Seismol. Soc.*
664 *Am.* **65**, 1613–1629.

665 Bodvarsson, G. (1970). Confined fluids as strainmeters, *J. Geophys. Res.* **75(14)**,
666 49–56.

667 Chadha, R. K., H. J. Kuempel and M. Shekar (2008 I). Reservoir triggered seismicity
668 (RTS) and well water level response in the Koyna-Warna region, India.
669 *Tectonophysics* **456**, 94–102.

670 Chadha, R. K., C. Singh, and M. Shekar (2008 II). Transient changes in well-water
671 level in bore wells in western India due to the 2004 Mw 9.3 Sumatra
672 Earthquake, *Bull. Seismol. Soc. Am.* **98**, 2553–2558.

673 Cooper, H. H., Jr., J. D. Bredehoeft, I. S. Papadopoulos, and R. R. Bennett (1965). The
674 response of well-aquifer systems to seismic waves, *J. Geophys. Res.* **70**,
675 3915–3926.

676 Doan, M. L., E. E. Brodsky, R. Prioul and C. Signer (2006). Tidal analysis of
677 borehole pressure-A tutorial, *Schlumberger Research report*, P35.

678 Elkhoury, J. E., E. E. Brodsky and D. C. Agnew (2006). Seismic waves increase

679 permeability, *Nature* **411**, 1135–1138.

680 Elkhoury, J. E., A. Niemeijer, E. E. Brodsky, and C. Marone (2011). Laboratory
681 observations of permeability enhancement by fluid pressure oscillation of in situ
682 fractured rock, *J. Geophys. Res.* **116**, B02311.

683 Green, D. H. and H. F. Wang (1986). Fluid pressure response to undrained
684 compression in saturated sedimentary rock, *Geophysics* **51(4)**, 948–956.

685 Geballe, Z. M., C.-Y. Wang, and M. Manga (2011). A permeability-change model for
686 water-level changes triggered by teleseismic waves, *Geofluids* **11**, 302–308.

687 Gassmann, F. (1951). Über die elastizität poröser medien, *Vierteljahrsschrift der*
688 *Naturforschenden Gesellschaft in Zürich*, **96**, 1–23.

689 Huang, F.-Q. (2008). *Response of Wells in Groundwater Monitoring Network in*
690 *Chinese Mainland to Distant Large Earthquakes*, [Ph.D Dissertation] Institute
691 of Geophysics, China Earthquake Administration, Beijing, pp. 47, (in Chinese).

692 Hsieh, P., J. Bredehoeft, and J. Farr (1987). Determination of aquifer transmissivity
693 from earthtide analysis, *Water Resour. Res.* **23**, 1824–1832.

694 Khan, S. A., and H. G. Scherneck (2003). The M2 ocean tide loading wave in Alaska:
695 vertical and horizontal displacements, modelled and observed, *J. Geodesy* **77**,
696 117–127.

697 Kayen, R. E., E. Thompson, D. Minasian, R. E. S. Moss, B. D. Collins, N. Sitar, D.
698 Dreger, G. A. Carver (2004). Geotechnical reconnaissance of the 2002 Denali
699 Fault, Alaska, Earthquake. *Earthq. Spectra* **20**, 639–667.

700 King, C.-Y., S. Azuma, G. Igarashi, M. Ohno, H. Saito, and H. Wakita (1999).

701 Earthquake-related water-level changes at 16 closely clustered wells in Tono,
702 central Japan, *J. Geophys. Res.* **104**, 13,073–13,082.

703 Kano, Y., Yanagidani, T., (2006). Broadband hydroseismograms observed by closed
704 borehole wells in the Kamioka mine, central Japan: Response of pore pressure
705 to seismic waves from 0.05 to 2 Hz, *J. Geophys. Res.*, **111**, 1–11.

706 Linde, A. T., I. S. Sacks, M. J. S. Johnston, D. P. Hill, and R. G. Bilham (1994).
707 Increased pressure from rising bubbles as a mechanism for remotely triggered
708 seismicity, *Nature* **371**, 408–410.

709 Liu, W. Q., M. Manga (2009). Changes in permeability caused by dynamic stresses in
710 fractured sandstone, *Geophys. Res. Lett.* **36**, L20307.

711 Liu, C., M. W. Huang, and Y. B. Tsai (2006). Water level fluctuations induced by
712 ground motions of local and teleseismic earthquakes at two wells in Hualien,
713 eastern Taiwan, *TAO* **17**, 371–389.

714 Liu, L.B., E. Roeloffs, and X. Y. Zheng (1989). Seismically induced water level
715 fluctuations in the WaliWell, Beijing, China, *J. Geophys. Res.* **94**, 9453–9462.

716 Liu, Y. R., and H. M. Tang (1998). *Rock Mass Mechanics*, Press of China University
717 of Geosciences, Beijing, 214 pp.

718 Lu, Y. Z., S. L. Li, Z. H. Deng, H. W. Pan, S. Che, and Y. L. Li (2002). *Seismology*
719 *Analysis and Prediction System Based on GIS (Mapseis Software)*, Chengdu
720 Map Press, Chengdu, China, 232.

721 Magara, K. (1978). Compaction and fluid migration, *Practical petroleum geology*.
722 Amsterdam: Elsevier Scientific Publishing Company, 1–319.

- 723 Nur, A., and J. Byerlee (1971). An exact effective stress law for elastic deformation of
724 rocks with fluids, *J. Geophys. Res.* **76**, 6414–6419.
- 725 Rice, J. R., and M. P. Cleary (1976). Some basic stress diffusion solutions for fluid-
726 saturated elastic porous media with compressible constituents, *Rev. Geophys.* **14**,
727 227–241.
- 728 Roeloffs, E. A. (1996). Poroelastic techniques in the study of earthquakes-related
729 hydrologic phenomena, *Adv. Geophys.* **37**, 135–195.
- 730 Roeloffs, E. A. (1998). Persistent water level changes in a well near Parkfield,
731 California, due to local and distant earthquakes, *J. Geophys. Res.* **103**, 869–889.
- 732 Sil, S. (2006). Response of Alaskan wells to near and distant large earthquakes,
733 Master's Thesis, University of Alaska Fairbanks, pp. 8–11.
- 734 Sil, S. and J. T. Freymueller (2006). Well water level changes in Fairbanks, Alaska,
735 due to the great Sumatra-Andaman earthquake, *Earth Planets Space* **58**,
736 181–184.
- 737 Terzaghi, K. (1925). Principles in soil mechanics, III. Determination of the
738 permeability of clay, *Engineering News Record.* **95**, 832–836.
- 739 Theo, H. S., M. H. Jacques, and C. C. Stephen (2002). Estimation of the poroelastic
740 parameters of cortical bone, *J. Biomech.* **35**, 829–835.
- 741 Wang, H. F. (2000). *Theory of linear poroelasticity with application to geomechanics*
742 *and hydrogeology*. Princeton University Press, Princeton, pp. 5–6.
- 743 Wang, H. F. (1993). Quasi-static poroelastic parameters in rock and their geophysical
744 applications, *PAGEOPH* **141**, 269–286.
- 745 Wang, C.-Y. (2007). Liquefaction beyond the near field, *Seismol. Res. Lett.* **78**,

746 512–517.

747 Wang, C.-Y., Y. Chia, P. L. Wang and D. Dreger (2009). Role of S waves and Love
748 waves in coseismic permeability enhancement, *Geophys. Res. Lett.* **36**, L09404.

749 Wang, C.-Y., and M. Manga (2010). *Earthquakes and Water, Series: Lecture Notes in*
750 *Earth Sciences*. Springer Press, Berlin, pp. 18–24.

751 Wang, C.-Y., and Y. Chia (2008). Mechanism of water level changes during
752 earthquakes: Near field versus intermediate field, *Geophys. Res. Lett.* **35**,
753 L12402.

754 Walters, R. A. and D. G. Goring (2001). Ocean tides around New Zealand, *New Zeal.*
755 *J. Mar. Fresh.* **35**, 567–579.

756 Wu, H. Z., L. Y. Fu and H. K. Ge (2010). Quantitative analysis of basin-scale
757 heterogeneities using sonic-log data in the Yanchang Basin, *J. Geophys. Eng.* **7**,
758 41–50.

759 West, M., J. Sanchez, and S. McNutt (2005). Periodically-triggered seismicity at Mt.
760 Wrangell volcano following the Sumatr earthquake, *Science*, **308**, 1144–1146.

761 Xue, Y.-Q. (1986). *Groundwater dynamics*. Geology Press, Beijing, pp. 16, (in
762 Chinese).

763 Zhang, Y., F. Q. Huang, and G. J. Lai (2009). Research on Skempton's coefficient *B*
764 based on the observation of groundwater of Changping station, *Earthq. Sci.* **22**,
765 631–638.

766 Zhang, Y., and F. Q. Huang (2011). Mechanism of Different Coseismic Water-Level
767 Changes in Wells with Similar Epicentral Distances of Intermediate Field, *Bull.*

768 *Seismol. Soc. Am.* **101**, 1531–1541.

769 Zeng, R. S. (1984). *Solid Geophysics Introduction*, Press of Science, Beijing, pp. 9,
770 (in Chinese).

771 Zhao L. F., X. B. Xie, W. M. Wang, and Z. X. Yao (2008). Regional Seismic
772 Characteristics of the 9 October 2006 North Korean Nuclear Test, *Bull. Seismol.*
773 *Soc. Am.* **98**, 2571–2589.

Appendix: An approach to Skempton's coefficient B based on the poroelastic theory

774 Skempton's coefficient B is a significant pore-fluid parameter in poroelastic
775 theory. A poroelastic material consists of an elastic matrix containing interconnected
776 fluid saturated pores. Fluid saturated crust behaves as a poroelastic material to a good
777 degree of approximation.

778 Rice and Cleary (1976) summarized the following equations for a linearly elastic
779 isotropic porous medium (they are the building blocks of the poroelastic theory):

$$780 \quad 2G\varepsilon_{ij} = \sigma_{ij} - \frac{\nu}{1+\nu} \sigma_{kk} \delta_{ij} + \frac{3(\nu_u - \nu)}{B(1+\nu)(1+\nu_u)} p \delta_{ij}, \quad (\text{A1})$$

$$781 \quad m - m_0 = \frac{3\rho(\nu_u - \nu)(\sigma_{kk} + 3p/B)}{2GB(1+\nu)(1+\nu_u)}. \quad (\text{A2})$$

782 Here $m - m_0$ is the change of the fluid mass, ε_{ij} is the strain tensor, σ_{ij} is the stress
783 tensor, δ_{ij} is the Kronecker delta function, G is the shear modulus, ρ is the
784 density of the fluid, B is the Skempton's coefficient, p is the pore pressure, ν is
785 the Poisson's ratio, and ν_u is the "undrained" Poisson's ratio. Rice and Cleary (1976)
786 describe equation (A1) as a stress balance equation and equation (A2) as a mass
787 balance equation.

788 For the undrained condition, the poroelastic effect on the crust can be obtained
789 by putting $m - m_0 = 0$ in equation (A2) to obtain

$$790 \quad p = -B\sigma_{kk} / 3 \quad \text{or} \quad \Delta p = -B\Delta\sigma_{kk} / 3. \quad (\text{A3})$$

791 Equation (A3) indicates that, in the undrained condition, the change in fluid pressure
792 (Δp) is proportional to the change in mean stress ($\Delta\sigma_{kk} / 3$). This is the mechanism of
793 water level changes for poroelastic material. ($p = \rho gh$, where h is the water column
794 height, g is the acceleration due to gravity and ρ is the density of water).

795 According to equation (A3), Skempton's coefficient B can be qualitatively
796 defined: In the undrained condition, B is the ratio of the induced pore pressure divided
797 by the change in mean stress (Wang, 2000). B governs the magnitude of water-level
798 changes due to an applied stress because pore pressure is directly proportional to
799 water level. The value of B is always between 0 and 1. When B is 1, the applied stress
800 is completely transferred into changing pore pressure. When B equals 0, there is no
801 change in pore pressure after applying the stress. Thus a low value of B indicates the
802 stiff rock matrix that supports the load with low coupling to the fluid (Nur and
803 Byerlee, 1971). Laboratory studies indicate the value of B depends upon the fluid-
804 saturated pore volume of the sample (Wang, 2000).

805 Equation (A3) can be expressed in terms of tidal strain as well (Roeloffs, 1996):

$$806 \quad \Delta h = -\frac{2GB(1 + \nu_u)}{3\rho g(1 - 2\nu_u)} \Delta \varepsilon_t. \quad (\text{A4})$$

807 Equation (A4) shows that water level changes proportionally in a poroelastic material
808 under the influence of tidal strain (ε_t). Here, Δh is the change in height of water
809 level, and $\Delta \varepsilon_t$ is the corresponding tidal strain change (Sil, 2006).

810 From equation (A4) we obtain:

$$811 \quad B = -\frac{3\rho g(1-2\nu_u)}{2G(1+\nu_u)} \frac{\Delta h}{\Delta \varepsilon_t}. \quad (A5)$$

812 With equation (A5), we obtain the value of B with water level and tidal strain.
 813 However, the calculation must be on the strict premise of the undrained condition (the
 814 good correlation between the water level and the tidal strain) and should not be
 815 influenced by the other factors.

816 For the effect of the solid tide on the crust, when the wavelength of the tidal
 817 strain is much larger than the size of the aquifer, we can suppose the aquifer system is
 818 undrained (Huang, 2008). So we can suppose the effect of the M_2 wave in the crust
 819 can meet the undrained condition (Zhang *et al.*, 2009). In addition, those wells can
 820 record clear tidal strains and thus, because we calculate the phase lags between the
 821 water levels and the tidal strains are small, the wells can readily meet the undrained
 822 condition. In the M_2 - wave frequency domain, the water level and the tidal strain
 823 show a good correlation; Furthermore, the M_2 wave is hardly influenced by
 824 atmospheric pressure. We therefore distill the frequency domain of the M_2 wave
 825 from the water level and the tidal strain by using band-pass filter (the frequency of the
 826 M_2 wave is $2.23636 \times 10^{-5} \text{ HZ}$) to calculate the Skempton's coefficient B . By
 827 converting the frequency domain of the M_2 waves (obtained from the water level and
 828 the tidal strain), by inverse fast Fourier transform and adjusting their phases (using the
 829 least-square fit and putting the results into equation (A5)), we can finally derive B .
 830 (More details of the method are explained in Zhang *et al.*, 2009). All the Water-level
 831 observations come from the sensor of water level, while tidal strain data are calculated
 832 via Mapseis software (see Data and Resources section). One thing needs to be
 833 clarified: We haven't applied the static equations directly to relate pore pressure

834 changes to seismic waves. We use those static equations for the impact of the tidal
835 strain on the aquifer medium before and after the Wenchuan earthquake, so as to
836 obtain the pre- and post- earthquake Skempton's coefficient B (those two periods can
837 be recognized as two independent quasi-static processes), so the poroelastic static
838 equations can be applied.

Studies of mechanisms for water level changes induced by teleseismic waves

1 Yan Zhang¹, Li-Yun Fu¹, Fuqiong Huang², Lian-feng Zhao¹, Yuchuan Ma², Bo Zhao²,
2 and Benxun Su¹

3 1. Key Laboratory of the Earth's Deep Interior, Institute of Geology and Geophysics,
4 Chinese Academy of Sciences, No. 19, Beitucheng Western Road, Beijing 100029,
5 China. E-mail: eve_041744@163.com; lfu@mail.iggcas.ac.cn

6 2. China Earthquake Networks Center, No. 5, Sanlihenanhang Avenue, Beijing100036,
7 China.

Abstract

8 The M_s 8.0 Wenchuan earthquake of May 12, 2008 induces large-amplitude
9 water level changes at intermediate and far fields (epicentral distance >1.5 fault
10 rupture length) in Chinese mainland. Although many hydrologic changes induced by
11 teleseismic waves have been reported, the mechanisms responsible for the changes
12 still remain unclear. We invoke Skempton's coefficient B and effective pressure in this
13 paper to explain those co-seismic water level changes documented in the intermediate
14 and far fields. The most used "enhanced permeability with a rapid redistribution of
15 pore pressure induced by removing loose particals from fractures by teleseismic
16 waves" can not be applied to explain all those coseismic water level changes in this
17 study. From our research we find some of those abrupt coseismic water level changes,
18 for which the variation of the co-seismic water level, and the effective pressure
19 preserve consistent (all increase or all decrease) are found to favor the consolidation
20 (porosity decrease) / dilatation (porosity increase) induced by the shaking of

21 teleseismic waves. Most of those wells have relatively high permeabilities attributing
22 to the shales in the aquifer lithologies. While the other part of those coseismic water
23 level changes (the variation of the co-seismic water level keeps inconsistent with the
24 variation of effective pressure), can be explained with the enhanced permeability with
25 a rapid redistribution of pore pressure, which is caused by fracture clearing or
26 overcoming the capillary entrapment in porous channels of the aquifer induced by the
27 shaking of teleseismic waves (most probably long period surface waves). Most of
28 those wells stay in basins or hollows, this kind of terrain inclines to lead to
29 heterogeneous pore pressure in close proximity.

Introduction

30 Various hydrologic responses to earthquakes have been documented ([Kayen *et al.*, 2004](#);
31 [Elkhoury *et al.*, 2006](#); [Sil and Freymueller, 2006](#); [Chadha *et al.*, 2008 II](#);
32 [Wang and Manga, 2010](#)), many occurred at great distances from the ruptured fault
33 where static stress changes are relatively small. Hydrologic changes induced by
34 teleseismic waves have been investigated in several studies of water wells ([Roeloffs,
35 1998](#); [Brodsky *et al.*, 2003](#); [Elkhoury *et al.*, 2006](#); [Geballe *et al.*, 2011](#)). Earthquake
36 induced water level changes at distant locations were reported after the Denali
37 earthquake ([Brodsky *et al.*, 2003](#); [Kayen *et al.*, 2004](#); [Sil and Freymueller, 2006](#)).
38 Seismic oscillations, due primarily to surface waves from distant events, occur in
39 some wells tapping highly transmissive aquifers ([Liu *et al.*, 1989](#); [Liu *et al.*, 2006](#)). [Sil
40 and Freymueller \(2006\)](#) developed an empirical relationship between water level
41 changes, epicentral distances and earthquake magnitude in the far-field. [Chadha *et al.*
42 \(2008 I\)](#) find wells appear to respond to regional strain variations and transient

43 changes due to distant earthquakes. [Liu and Manga \(2009\)](#) indicate that significant
44 water level changes can be driven at great distances by moderate-amplitude dynamic
45 (time-varying) stresses.

46 Several mechanisms have been proposed to explain these co-seismic changes in
47 water level. Fracture clearing and increased permeability caused by the
48 earthquake-induced dynamic stress have been widely used to explain most
49 documented far-field water level changes ([Brodsky *et al.*, 2003](#); [Elkhoury *et al.*, 2006](#);
50 [Wang and Chia, 2008](#); [Wang and Manga, 2010](#)). Overcoming the capillary
51 entrapment in porous channels is hypothesized to be one of the principal pore-scale
52 mechanisms by which natural permeability is enhanced by the passage of elastic
53 waves ([Beresnev, 2011](#)). Dynamic strain induced by the passage of seismic waves,
54 most probably long period surface waves might be the cause of water level changes in
55 the far-field ([West *et al.*, 2005](#); [Sil and Jeffrey, 2006](#); [Chadha *et al.*, 2008 II](#)). Other
56 proposed, but also unverified mechanisms include pore pressure increases caused by a
57 mechanism ‘akin to liquefaction’ ([Roeloffs, 1998](#)), shaking-induced dilatancy ([Bower
58 and Heaton, 1978](#)), increasing pore pressure through seismically induced growth of
59 bubbles ([Linde *et al.*, 1994](#)), and fracture of an impermeable fault ([King *et al.*, 1999](#)).
60 In addition, [Huang \(2008\)](#) observed the co-seismic water level increase may be
61 caused by the consolidation induced by the transmission of teleseismic waves in
62 Fuxin well. Experimental measurements of [Liu and Manga \(2009\)](#) indicate that
63 permeability changes (either increases or decreases) owing to dynamic stresses are a
64 reasonable explanation. [Wang *et al* \(2009\)](#) find that the groundwater flow associated
65 with S and Love waves may generate shear stress large enough to break up the flocs
66 in sediment pores and to enhance the permeability of aquifers.

67 In the present study, we use the Skempton’s coefficient B , the co-seismic water

68 level and the inferred effective pressure to explain the co-seismic water level changes
69 in the intermediate and far fields based on datasets from the Wenchuan earthquake in
70 the Chinese mainland. Using a poroelastic relation between water level and solid tide
71 ([Zhang et al., 2009](#)), we calculate the in-situ Skempton's coefficient B both pre and
72 post earthquake (which are two independent quasistatic processes). From the research
73 we find: Consolidation/dilatation induced by shaking of teleseismic waves, may
74 account for the mechanism of those abrupt coseismic water level changes, for which
75 variations of co-seismic water level and effective pressure preserve uniformity. Most
76 of those wells have relatively high permeabilities attributing to the shales in the
77 aquifer lithologies. While, the other part of those coseismic water level changes, for
78 which the co-seismic water level and the effective pressure change with inconformity
79 (most of those wells stay in basins or hollows), may be explained with the increased
80 permeability caused by teleseismic waves, which in turn lead to the redistribution of
81 pore pressures. Compare the occurrence time of water level changes with the arrival
82 time of surface waves in two stations, we find the co-seismic water level changes are
83 induced by the long period surface waves.

Selection Principles and Observations

84 Large numbers of stations with co-seismic water level changes induced by
85 M_s 8.0 Wenchuan earthquake have been collected in the intermediate and far fields
86 (>1.5 fault-rupture lengths). Most of those water level changes in this area can not be
87 induced by the change of the static strains, which are extremely tiny ([Zhang and](#)
88 [Huang, 2011](#)). We selected those co-seismic water level changes with distinct
89 amplitude (tiny or obscured co-seismic water level changes have been excluded). In

90 order to calculate the pre- and post- earthquake B values, water level data in stations
91 should not be long-time missing or be influenced by other factors, such as pumping or
92 other disturbances, and the data should be long enough (at least with a 10-day
93 continuous data before and after the earthquake respectively), so that we can use the
94 least-square fit to calculate B (Appendix). In addition, the oceanic tides has been
95 known to have an effect several tens of kilometers away from the seashore (Beaumont
96 and Berger, 1975). The deformation caused by ocean tide loading is difficult to
97 calculate, these tides appear with the same frequencies as the solid earth effects (Khan
98 and Scherneck, 2003), and the tides are strongly affected by the complicated
99 topography around the seashore (Walters and Goring, 2001), so we can't simply to
100 calculate the oceanic tides by theory models. Besides, there are no public software to
101 calculate the China national offshore ocean tides, so we have to delete those wells (4
102 wells: Hejiazhuang, Huanghua, Wafangdianloufang and Yongchun) which may be
103 influenced by the ocean tides seriously. Bearing those rules in mind, we find 11
104 stations (well a to well k (Figure 1)) can be chosen during the Wenchuan earthquake
105 (Table 1).

106 Detailed basic information of each well are show in Table 1 , including well
107 depth, well diameter, aquifer lithology, and geological structure. However, diameter of
108 well g, h and j can not be found. All the water level recording instruments in those
109 wells (well a to well k) are digital, they are LN-3A digital water level instrument
110 (except for Mile well it uses LN-4A digital water level instrument, and Fuxin well
111 uses the SQ digital water level instrument), with the observation accuracy $\leq 0.2\%$ F.S. ,
112 and the sampling rate of 1/min, the resolution ratio is 1mm. We use the Mapseis
113 software (Lu *et al.*, 2002) to calculate the tidal strain data (hourly data). In order to
114 keep in accordance, both the water level and the tidal strain use the hourly data when

115 calculating the Skempton's coefficient B .

Intermediate and Far Field Analysis

Assumptions of shear modulus and Poisson's ratio and the calculation of Skempton's coefficient B

116 Calculations are performed using $\rho = 1000 \text{ kg/m}^3$, $g = 9.8 \text{ m/s}^2$, and $\nu_u = 0.29$
117 according to equation (A5) (Appendix). We suppose the undrained Poisson's ratio
118 $\nu_u = 0.29$ both pre and after earthquake, and this kind of assumption is always used
119 to simplify calculation issues of rocks near the crust (Zeng, 1984). In addition, based
120 on the poroelastic theory, and limited to isotropic conditions, Theo *et al.*(2002) aim to
121 determine the elastic material constants of the solid matrix with two level of porosities.
122 As it is not possible to experimentally determine the elastic material constants of the
123 solid matrix at these levels, a theoretical approach is presented, based on experimental
124 data taken from literature. They find different porosities lead to different values of
125 elastic modulus. Their results indicate that the variation extents of Skempton's
126 coefficient B and the bulk modulus are much larger than the drained and undrained
127 poisson's ratios (variation extent of B : 6.3% ; variation extent of K : 7.96% variation
128 extent of ν_u : 0.3%). So we can approximately assume that compared to the
129 variations of Skempton's coefficient B , the change of the undrained poisson's ratio
130 can be neglected before and after the earthquake.

131 Gassmann (1951) predicted that the effective shear modulus would be
132 independent of the saturating fluid properties (the shear modulus is a constant) in the
133 undrained isotropic poroelastic media. As studied by Berryman (1999) and Berryman
134 and Wang (2001), the theory applies at very low frequencies. At high enough
135 frequencies (especially in the ultrasonic frequencies), as the numerical simulation of

136 [Berryman and Wang \(2001\)](#) shows (based on the effective medium theory, and use a
137 complete set of poroelastic constants for drained Trafalgar shale), with the increase of
138 Skempton's coefficient B , the bulk modulus changes by as much as 100% in this
139 example, whereas the shear modulus changes by less than 10%, and other rock
140 examples also show similar results ([Berryman and Wang, 2001](#)). As discussed above,
141 we can know: It is obvious that the change of shear modulus G is tiny, and even can
142 be neglected (both in the drained or undrained cases) as compared with the change of
143 Skempton's coefficient B . In this paper we suppose, shear modulus of well aquifer
144 systems will not change after affected by the seismic waves (the frequencies of
145 seismic waves are much lower than the ultrasonic frequencies, so the change of the
146 shear modulus will be neglectable compared to the change in B value).

147 We apply the B -calculation method ([Appendix](#)) to those well-picked stations.
148 The pre-and post-earthquake B values are respectively obtained from May 1, 2008 to
149 May 11, 2008, and from May 13, 2008 to May 24, 2008 ([Figure 2](#)).

Undrained Skempton's coefficient B as a function of effective pressure

150 When the aquifer be consolidated, the effective pressure (effective pressure =
151 confining pressure - pore pressure) will increase, while a dilation is in accordance to
152 the decrease of effective pressure. [Blocher et al. \(2009\)](#) measured the relationship
153 between Skempton's coefficient B and effective pressure based on the laboratory
154 experiment. The in-situ aquifer of those wells (well a~k) we studied are under
155 lithostatic pressures for a long time and also be affected by the transmission of
156 seismic waves for countless times, the situation is much similar to those well bedrocks
157 be applied on repeated pressure cycles. So the situation will be much similar to the
158 last several ramps (apply more than once pressure cycles on the rock) rather than the

159 first ramp (apply the first pressure cycle on the rock, during which a possible
160 dissolution of gas in the fluid of an incompletely saturated sample happened) in the
161 experiment of Blocher *et al.* (2009), and the isotropic Skempton's coefficient B will
162 increase/decrease with the increase/decrease of effective pressure (when the effective
163 pressure is less than ~ 4 Mpa), while B will decrease with the increase of effective
164 pressure (when the effective pressure is larger than ~ 4 Mpa). Although these results
165 obtained from sandstone, because of the lack of the laboratory experiment study of
166 those specific rocks, we assume the results can be applied to the bedrock of all those
167 wells studied in this paper.

168 In order to compare with the experiment results, we have to estimate the
169 effective pressure of each well. Pore pressure response to gravitational loading is
170 similar to tectonic loading and can also be treated as a poroelastic problem (Green and
171 Wang, 1986). Depths of those wells analyzed in this paper are all less than 1km
172 (Table 1). W-1 well lies in Yanchang basin of Gansu province, Yanchang basin is a
173 deep basin with Paleozoic sediments (Wu *et al.*, 2010). The "pressure - depth"
174 relation of well W-1 (Figure 3a) is similar to other wells in the Chinese mainland. So
175 we assume those results could be applied to these wells we studied (well a \sim k) since
176 we lack the "pressure-depth" predictions of these wells. We calculate the effective
177 pressure of W-1 well (effective pressure approximately equals to lithostatic pressure
178 minus pore fluid pressure) (Figure 3b), and estimate the range of the effective
179 pressure of these wells we studied according to the well-depth (Table 1).

180 We calculated the change of pore pressure in each well ($\Delta P_p = \rho g \Delta h$), together
181 with the range of the effective pressure, the variation trend of Skempton's coefficient

182 B , and the B -effective pressure relation obtained by the experiment of Blocher *et al.*
183 (2009), we can infer the variation quantity of the effective pressure in each well
184 (Table 2, Table 3). When the range of the effective pressure lies in 0-3 Mpa (most of
185 the wells), the increase/decrease of B accompanied with the increase/decrease of
186 effective pressure. When the range of effective pressure >5 Mpa, the
187 increase/decrease of B accompanied with the decrease/increase of effective pressure
188 Blocher *et al.* (2009), only the effective pressure of Jurong well (well f) lies in this
189 range (Table 2).

Mechanism analysis

190 Till now, fracture clearing (unclogging) and increased permeability has been
191 used to explain most of those coseismic water level changes in the far field (Brodsky
192 *et al.*, 2003; Wang, 2007; Wang and Manga, 2010). Since pore-pressure heterogeneity
193 may be the norm in the field, an enhancement of permeability among sites of different
194 pore pressure may cause pore pressure to spread (Roeloffs, 1998; Brodsky *et al.*, 2003;
195 Wang, 2007; Wang and Manga, 2010). Analysis of well response to tidal forcing
196 before and after an earthquake has provided strong evidence that earthquakes can
197 enhance permeability (Elkhoury *et al.*, 2006). In this study, we calculate the change of
198 Skempton's coefficient B and effective pressure, however, we can not use the
199 enhanced permeability theory to explain all those coseismic water level changes. And
200 we find the other part of water level changes may favor the consolidation or dilatation
201 induced by teleseismic waves (about 58.3% of all those wells analyzed in this paper
202 favor this explanation).

203 Permeability will increase/decrease, which is mostly related to the
204 increase/decrease of porosity (Xue, 1986). As explained by rock mechanics the same
205 porosity always corresponding to the same effective pressure (Terzaghi, 1925;

206 [Magara, 1978](#)). From that we can know porosity and permeability are all directly
207 connected with effective pressure, and they will decrease with the increase of the
208 effective pressure ([Blocher et al., 2009](#)).

209 We can summarize the variation of effective pressure ($P_{eff} = P_c - P_p$) in two ways:
210 (P_c confining pressure, P_p pore pressure, and P_{eff} effective pressure)

211 A) Pore pressure P_p keeps constant, the change of effective pressure P_{eff}
212 induced by the change of confining pressure P_c .

213 There are two states ([Table 4](#)): (a1) Confining pressure increases (pore pressure
214 not change), then effective pressure increases, the porosity will decrease (a process of
215 consolidation or squeeze), and water level / pore pressure will increase; (a2)
216 Confining pressure decreases (pore pressure not change), then effective pressure
217 decreases, the porosity will increase (a process of dilatation), and water level / pore
218 pressure will decrease. (a1), (a2) can be summarized as a mechanism of water level
219 change induced by consolidation or dilatation, and water level changes in accordance
220 with the change of effective pressure (all increase or all decrease) in this case.

221 B) Confining pressure P_c keeps constant, the change of effective pressure P_{eff}
222 induced by the change of pore pressure P_p .

223 There are two states ([Table 4](#)): (b1) Pore pressure/ water level decreases
224 (Confining pressure not change), then effective pressure increases, the porosity will
225 decrease (a process of water level flows out of the well to a place with a relatively
226 lower pore pressure); (b2) Pore pressure/ water level increases (Confining pressure

227 not change), then effective pressure decreases, the porosity will increase (a process of
228 water level flows into the well from a place with a relatively higher pore pressure).
229 (b1), (b2) can be summarized as a mechanism of water level change induced by
230 increased permeability with a rapid redistribution of pore pressure (this is the most
231 used mechanism for far-field coseismic water level changes), and water level changes
232 opposite to the change of effective pressure in this case.

233 As show in below (part 4.1 and part 4,2), we use two mechanisms to explain
234 those coseismic water level changes.

Coseismic water level change induced by increased permeability followed by a rapid redistribution of pore pressure

235 The effective pressure range of well h, and i is 0 ~ 3 MPa ([Table 2](#)). According
236 to the laboratory experiment of [Blocher *et al* \(2009\)](#), the increase of effective pressure
237 accompanied with the increase of Skempton's coefficient B in this range. Water levels
238 (pore pressure) decrease accompanied with the increase of effective pressures in well
239 h, and i ([Table 2](#)). Since pore-pressure heterogeneity may be the norm in the field, an
240 enhancement of permeability among sites of different pore pressure may cause pore
241 pressure to spread ([Roeloffs, 1998](#); [Brodsky *et al.*, 2003](#); [Wang, 2007](#); [Wang and](#)
242 [Manga, 2010](#)). Pore-pressure of the two wells may be higher than the close proximity
243 before the earthquake, an enhancement of permeability incurred by (for example)
244 overcoming the capillary entrapment in porous channels induced by the passage of
245 elastic waves will decrease the pore-pressure in wells (the pore-pressure will shift to
246 other places), and water level will decrease. Then the effective pressure will increase
247 accompanied with the decrease of pore-pressure (water level), so the Skempton's

248 coefficient B increases (which indicates the stiff rock matrix could with a higher
249 coupling to the fluid) in well h and i (Table 2).

250 The depth of well f (889.18 m) is larger than other wells, and the effective
251 pressure range of this depth is 8 ~ 10 MPa (Table 2). According to the laboratory
252 experiment of Blocher *et al* (2009), the decrease of effective pressure accompanied
253 with the increase of Skempton's coefficient B in this range. Water level increases with
254 the decrease of effective pressure (increase of Skempton's coefficient B) in well f, this
255 should be explained with the increased permeability. Pore-pressure of well f may be
256 lower than the close places before the earthquake, an enhancement of permeability
257 will increase the pore-pressure in this well (the pore-pressure (water level) may shift
258 from other places), and water level (pore pressure) will increase. Then the effective
259 pressure will decrease accompanied with the increase of pore-pressure (water level),
260 supposing the confining pressure not change. As explained by Blocher *et al* (2009),
261 with the increase of effective pressure (reaches larger than 5 Mpa), the decrease of
262 the Skempton's coefficient results from the change of the pore-geometry, which leads
263 to a higher bulk modulus of the sample. Pore throats and microcracks were closed,
264 and the stiff rock matrix could with a lower coupling to the fluid, so the Skempton's
265 coefficient B decreases. And this is an reversible process (after they raised the
266 confining pressure from 5 to 50 Mpa, they lowered the confining pressure form 50 to
267 5 Mpa, and also obtained the similar results), so when the effective pressure decreases
268 (not lower than 5 Mpa), the closed pore throats and microcracks will be opened and
269 turn larger under the effect of pore pressure, the stiff rock matrix could with a higher
270 coupling to the fluid in well f, leading to the increase of Skempton's coefficient B .

271 The local geological structure of each well is important (Table 1). We find that
272 most of those wells in which the coseismic water level changes can be explained with

273 “ the enhanced permeability with a rapid redistribution of pore pressure” stay in
274 basins or in hollows (well f, h, i and Fuxin). The terrains of those wells incline to lead
275 to heterogeneous pore pressures in close proximities (possibly attributed to different
276 altitudes).

Coseismic water level change induced by consolidation or dilatation

Coseismic water level change induced by dilatation

277 For well g, j and k, the effective pressure range is 0 ~ 3 MPa, effective pressure
278 will increase/decrease accompanied with the increase/decrease of Skempton’s
279 coefficient B during this range (Blocher *et al.*, 2009). Water levels (pore pressures) of
280 well g, j and k decrease, accompanied with the decrease of effective pressures [and
281 decrease of Skempton’s coefficient B (which indicates the stiff rock matrix could with
282 a lower coupling to the fluid)], which can not be explained with the increased
283 permeability followed by the rapid pore pressure redistribution between the well and
284 the places near the well. Whereas, this could be explained with the state (a2)
285 Confining pressure decreases (pore pressure not change), then effective pressure
286 decreases, the porosity will increase (a process of dilatation), and water level / pore
287 pressure will decrease.

288 The spreading of teleseismic waves may cause dilatation of the aquifer medium,
289 which can broaden the porosities (the permeability will increase) and give birth to
290 new fractures, and the effective pressure will reduce (in wells: g, j and k) leading to
291 the decrease of Skempton’s coefficient B . This explanation is similar to the
292 mechanism of shaking-induced dilatancy (Bower and Heaton, 1978).

Coseismic water level change induced by consolidation

293 For well a, b, c, and d, the effective pressure range is approximately 0 ~ 3 MPa,
294 effective pressure will increase/decrease accompanied with the increase/decrease of
295 Skempton's coefficient B (Blocher *et al.*, 2009). Water level (pore pressure) of well a,
296 b, c, and d increase, accompanied with the increase of effective pressure [and increase
297 of Skempton's coefficient B (which indicates the stiff rock matrix could with a higher
298 coupling to the fluid)], which also can not be explained with the increased
299 permeability followed by the rapid pore pressure redistribution between the well and
300 the place near the well. Whereas, this could be explained with the state (a1)
301 "Confining pressure increases (pore pressure not change), then effective pressure
302 increases, the porosity will decrease (a process of consolidation or squeeze), and
303 water level / pore pressure will increase". This mechanism is very similar to the
304 explanation of the laboratory experiment of Liu and Manga (2009). From their
305 laboratory experiment, they find that: in general, permeability/porosity decreases after
306 shaking. They measured the evolution of permeability in fractured sandstone in
307 response to repeated shaking under undrained conditions, and set the frequency and
308 amplitude of the imposed shaking to be representative of those that cause distant
309 hydrological responses. As they explained: Dynamic strains cause time varying fluid
310 flow that can redistribute particles within fractures or porespace, and can allow
311 particles to move away from regions where they hold pore spaces open, and are
312 expected to accumulate and get trapped at the narrowest constrictions along flow
313 paths, and hence allow a consolidation (contraction) of the sample, which can lead to
314 a higher coupling between the stiff rock matrix and the fluid. Their result just supports
315 our mechanism analysis. It implies that teleseismic waves can cause a consolidation
316 of well aquifer and cause the increase of effective pressure (decrease of permeability
317 and porosity), which is in accordance with the increase of co-seismic water levels

318 accompanied with the increase of Skempton's coefficient B in well a, b, c, and d.

Examples support far field water level increases induced by consolidation

319 We analyze the mechanism of the coseismic water level changes induced by
320 consolidation incurred by teleseismic waves in above. However, water level increases
321 induced by consolidation in the far field is not the mainstream view. It is necessary to
322 give some examples which can support far field water level increases induced by
323 consolidation.

324 [Huang \(2008\)](#) find that: the water level increase in Fuxin well (1409.98 km away
325 from Wenchuan, the well depth is 60.74 m, stiff Granite with a little basalt is the
326 bedrock and we assume the shear modulus = 60 Gpa) is induced by the increase of
327 volume strain (consolidation) ([Figure 4a](#)). In the Chinese mainland, Fuxin is the only
328 well in which there are observations of volume strain and water level in a specific
329 aquifer medium, and both of them show obvious co-seismic responses to Wenchuan
330 earthquake. There are clear and obvious effects of tidal strain and atmospheric
331 pressure in the water level and volume strain, which indicates Fuxin is a terrific
332 artesian well. This well has not be chosen in the above analysis because there is an
333 abrupt large-amplitude increase in the water level, which starts from 11 p.m. May 22,
334 2008 (we can not find any interference of this abrupt increase according to the daily
335 records of Fuxin station), and we can just use a shorter time period to calculate the
336 post-earthquake B value, which may cause a little impact on the precise of B . The
337 calculation is performed based on the M_2 wave distilled from the water level and
338 the tidal strain (pre-earthquake: from May 1, 2008 to May 11, 2008, post-earthquake:
339 from May 13, 2008 to May 22, 2008 ([Figure 4b](#))). (The large-step abrupt water level
340 increase starts from 09 p.m. May 22, 2008 ([Figure 4c](#)), which may cause large impact

341 on the detrend process and influence the calculation result, so we discard these data).
342 From [Figure 4a](#), we can see the coseismic water level increase is induced by the
343 change of the volume strain, which indicates the well aquifer has been consolidated.
344 The depth of Fuxin well is 60.74 m, and we can assume the range of the effective
345 pressure is 0~3Mpa ([Table 2](#)), from the change of the pre- and post- earthquake B
346 ([Figure 4b](#)), we may infer the consolidation may be very extreme, accompanied with
347 the coseismic water level increase it could cause an extra pressure, which overcomes
348 the capillary entrapment in porous channels of the aquifer or incurs a fracture
349 clearing and bring in the increase of the permeability, then water flow in from other
350 places with a higher pressure, which lead to the decrease of the Skempton's
351 coefficient B with the decrease of the effective pressure, and the water level increases
352 more gradually (corresponding to the state (b2)). Finally with the further enhancement
353 of the permeability (increase of the porosity), a permanent deformation could be
354 induced, so there is an abrupt increase in the water level in 22 May, and remain in a
355 relatively high level for several months([Figure 4c](#)). From the picture we can see it
356 may be in a drained condition after the abrupt large-amplitude water level increase,
357 because the water level fluctuates irregularly.

358 So we argue that water level increase induced by the consolidation incurred by
359 transmission of teleseismic waves is reasonable, and in a specific geology condition,
360 a consolidation with large enough energy may also lead to an enhanced permeability
361 by fracture clearing or by overcoming the capillary entrapment in porous channels.

Conclusion of coseismic water level changes induced by consolidation or dilatation

362 Water level increases/decreases accompanied with the increase/decrease of
363 effective pressure (and the increase/decrease of Skempton's coefficient B) in well a, b,

364 c, d, g, j, and k (the effective pressure range is approximately 0 ~ 3 MPa) (Table 3).
365 To our understanding, suppose the pressure not exceed a limitation (a permanent
366 deformation not happened), when the aquifer be consolidated/ dilatated, the mean
367 fracture width (the porosity and permeability) may decrease/increase with the
368 increase/decrease of the effective pressure, then the stiff rock matrix that supports the
369 load could with a higher/lower coupling to the fluid (Nur and Byerlee, 1971), and the
370 value of B will increase/decrease. Hence, shaking induced by the transmission of
371 teleseismic waves may cause consolidation/dilatation of the aquifer, and lead to the
372 increase/decrease of the water level. Figure 5 shows the relation between the change
373 of Skempton's coefficient B and the change of effective pressure in well a, b, c, d, g, j,
374 and k . Approximately, it displays a linear relation.

Well lithologic logs and permeability

375 As indicated by Wang *et al.* (2009) High transmissivity promotes uniform pore
376 pressure, thus there is a low probability of connecting to a reservoir of different
377 pressure. On the other hand, poor transmissivity can support heterogeneous pore
378 pressure in close proximity, thus there is a high probability of connecting to a
379 reservoir of different pressure. We show the well lithologic logs (borehole columnar
380 diagrams) in Figure 6. According to <China earthquake monitoring record series>
381 [which is written by different Subordinate units (earthquake administration of each
382 provinces and different institutions) of China Earthquake Administration, and
383 published in Beijing in different years by Seismological Press (in Chinese)], we can
384 only get the lithologic logs of well (a), (c), (d), (e), (f), (k) and Fuxin (Figure 6), the
385 pictures are designed already, some lithologic logs are explained in detail and some
386 are in shot. Shales display in Lithologic logs of well (c), (d) , (e) [Although there is no

387 obvious records of shales in the log of well e, according to the <China earthquake
388 monitoring record series> there are shales (may be a small quantity of shale) in the
389 matrix rock of well (e) (Table 1)] and (k), so the permeability of well (c), (d) , (e) and
390 (k) may be relatively larger, and they are well connected with the close places outside
391 the well, so there are little pressure differences between those wells and the places
392 around them, and water level tend not to flow into or out of those wells even if the
393 porosity/permeability increased by the shaking of telesismic waves. On the other hand,
394 there are no shales in the logs of well (a), (f) and Fuxin, and the permeability may be
395 relatively smaller, thus there is a high probability of connecting to a reservoir of
396 different pressure, and water level tend to flow into or out of those wells after the
397 porosity/permeability increased by the shaking of telesismic waves. Except for well
398 (a), those analyses of lithologic logs are in accordance with our above mechanism
399 analyses [well (c), (d), (e) and (k) favor the consolidation/ dilatation mechanism,
400 while well (f) and Fuxin are induced by the increased permeability followed by a
401 rapid redistribution of pore pressure]. Other factors may also influence the
402 permeability, such as the geometry of the well. However, we can not make further
403 study about that, since we lack the in situ tests and the detailed lithologic logs of other
404 wells.

405 Because we use the approximately median value of shear modulus G (Table 1) to
406 calculate the Skempton's coefficient B , it is hard to estimate the permeable extent of
407 the aquifer from the absolute value of B . Whereas, the variation tendency of B before
408 and after the earthquake is definite.

Wellbore storage effects

409 Tidal phase lags are caused by wellbore storage. "Wellbore storage" is the term
410 used to describe a lag of piezometer water level behind aquifer pressure resulting

411 from the need for water to flow into the borehole in order to equilibrate water level
412 with aquifer pressure. Wellbore storage effects are a function of the transmissivity
413 between the well and aquifer, in addition to the geometry of the well (Cooper *et al.*,
414 1965; Liu *et al.*, 1989; Kano and Yanagidani, 2006). Wellbore storage effects increase
415 (phase lags increase) as the transmissivity (and permeability) of the formation
416 decreases (Roeloffs, 1996; Doan *et al.*, 2006).

417 Most of those wells can record clear tidal strain and atmospheric pressure, and
418 according to the <China earthquake monitoring record series> they are well confined.
419 From Table 1 we can see the phase difference of water level and tidal strain of most
420 wells are 0, which mean good correlations between the water levels and the tidal
421 strains, and those wells are well confined and under the undrained condition. Hsieh *et*
422 *al.* (1987) indicates that: the computed O1 phase shift is subject to large uncertainty,
423 while the computed M2 phase shift is substantially more accurate. So we use the M2
424 wave to calculated the phase shift. Because we use the hourly data, we can not
425 identify the phase difference when it is less than 1 hour, and we neglected the
426 wellbore storage effects in those wells. Before and after the earthquake, if phase lags
427 remain the same, it indicates the permeability of the well aquifer keeps the same or
428 changes not much (the phase difference may be lees than 1 hour). Phase lags $\cong 1$
429 hour in well b, c, e, and Fuxin, and most of them are small, except well b, which may
430 be semi-confined. Thus, the validity of the calculated *B* values in well b may be a
431 little questionable. The phase lag of Fuxin well decreases after the earthquake (L1=2
432 hours, L2=1 hour), which indicates the permeability increases after the shakig of the
433 earthquake, this is in accordance with the mechanism analysis of the co-seismic water
434 level increase in Fuxin well.

435 Except for well e (Table 5), it is out of our expectation. Although there is no

436 obvious records of shales in the lithologic logs of well e, there are shales (may be a
437 small quantity of shale) in the aquifer lithology according to the <China earthquake
438 monitoring record series> (Table 1), and the permeability in well e may be relatively
439 high, so it connects well with the place outside, thus there is a low probability of
440 connecting to a place of different pressure. Phase lag increases (which indicates a
441 decrease in permeability) accompanied with the increase of water level in well e. In
442 our expectation, this situation should incur an increase in Skempton's coefficient B
443 (an increase in effective pressure), which indicates the aquifer be consolidated
444 (squeezed). However, the effective pressure (Skempton's coefficient B) decrease in
445 well e, this may be attributed to the fast decrease of water level after the earthquake
446 (Figure 2). Further researches need to be done so as to detect the mechanism more
447 clearly.

Discussion

The variation of porosity

448 Figure 3c shows, in general, the porosity decreases with the increase of depth,
449 however, when reach 3000m the effective pressure turns much larger (approximately
450 equals to 35 Mpa) than that in the depth of those wells (well a ~ k), the porosity still
451 persists relatively large, and changes with different depth. From Table 3 we can see,
452 the variations of effective pressure in well a, b, c, d, g, j and k are less than 0.01Mpa,
453 and from Figure 3b we know, variation of 0.01Mpa in effective pressure
454 approximately equals to variation of 1 meter in depth, as Figure 3c shows, the
455 variation of porosity is tiny during variation of 1 meter in depth. So this variation
456 extent of effective pressure is hard to induce permanent deformation of porosity.
457 However, in reality, the change of porosity may also connected with the formation

458 and the state of the rock matrix.

459 Furthermore, phase lags of well a, b, c, d, g, j and k keep constant before and
460 after the earthquake (change less than 1 hour) (Table 1), so we can infer, the porosity
461 (permeability) changes little after the earthquake. Because the phase lags
462 increase/decrease (wellbore storage effects increase/decrease) as the permeability
463 (porosity) of the formation decreases/increases (Roeloffs, 1996; Doan *et al.*, 2006).

464 So we can infer, the porosity of well a, b, c, d, g, j and k can persist despite being
465 reduced/enlarged due to the consolidation/dilatation induced by the passage of
466 teleseismic waves of M_s 8.0 Wenchuan earthquake.

Uncertainty of B coefficient

467 In order to study the uncertainty of B coefficient (error related to the
468 determination of B coefficient), we use Jurong well to show the variation of B during
469 a relatively long – time span (50 days before and after the Wenchuan earthquake)
470 (Figure 7). Skempton's coefficient B will change with the change of time. Because we
471 use the least square fit to calculate B , the value may be a little different when we use
472 different length of data, but the change tendency (increase or decrease of B) before
473 and after the earthquake will be constant. Furthermore, we can see the B value of
474 Jurong well recover to its initial value after about 30 days (Figure 7).

475 So, compared with the uncertainty in B value, variation of B due to the
476 earthquake is significant. The continuous of B will be influenced by lots of factors,
477 such as power off, aftershocks, and so on, so B -value series at large time scale is not
478 easy to obtain for each well.

Recovery of Water level

479 The recovery time of the water level is obscure, because most of those water

480 levels will not recover to the pre-earthquake heights during a relatively short time
481 span. So we should use much longer data to analyze it, and should discard all those
482 influences: such as aftershocks, atmospheric pressure (not all those wells have the
483 records of atmospheric pressure) tidal strain, pumping, power off, thounder and so on,
484 which needs lots of work, and we may study about it in future. In addition, we haven't
485 find any relation between water level changes and epicentral distances in those wells
486 studied in this paper, it is possible to investigate much more wells later, to study about
487 the relations.

The variation value of effective pressure

488 We calculat the change of pore pressure ($\Delta p_p = \rho g \Delta h$), and we can use the critical
489 state to help us to analyze the variation value of effective pressure in each well.

490 When the aquifer be consolidated/dilated, in the critical state, the pore pressure
491 keeps constant, the confinging pressure increases/decreases, then the effective
492 pressure increases/decreases, and at last transfers into the increase/decrease of pore
493 pressure (water level), and the system comes into an equilibrium state. So the change
494 of pore pressure can be attributed to the change of the effective pressure.

495 When the permeability increases, in the critical state, the confining pressure
496 keeps constant, the pore pressure (water level) increases (the well in a relatively low
497 pressure region before the earthquake) /decreases (the well in a relatively high
498 pressure region before the earthquake), then the effective pressure decreases/increases,
499 so the change of the effective pressure can be attributed to the change of pore
500 pressure.

501 However , the variation value of the effective pressure in each well may be

502 different from the value we calculate, because the critical state is an assumption ideal
503 state, and the transfer of stress may also relate with the formation and state of the
504 aquifer.

Compare with seismograms

505 There are 48 national stations recording the seismograms (event waveforms) in
506 the Chinese mainland (we can not obtain some of the regional seismograms because
507 of the authority limitation), however most of those stations are not in the same place
508 with stations which have the records of water level changes. Those stations (well a to
509 k) analyzed in our paper do not record seismograms. After comparison, generally we
510 may use the seismograms of 4 national stations to analyze the corresponding water
511 level observations, which are near those national stations (the distances between the
512 water level wells and the national seismogram stations are approximately less than
513 100km). However, there are deficiencies in the seismograms of station TIY and LZH
514 (TIY station is corresponding to well e (there are about 40.903 km between them);
515 LZH station is corresponding to well g (there are about 19.82 km between them)), so
516 we desgarded them. Finally, two seismograms can be used: the seismogram of SNY
517 (Shengyang) station is used to analyze Fuxin well (there are about 102.81 km between
518 them), and HEF (Hefei) station is corresponding to well k (there are about 91.57 km
519 between them). In addition, the geology conditions are very similar (the main matrix
520 rocks of Fuxin well and Shengyang station are both granite; Well k is in Chuhe river
521 major dislocation and Hefei-Dongguan fracture intersection).

522 There are only hourly water level data in Fuxin well (minute data observation

523 strats from 2009), so we can not use that to do precise comparison (in minute) with
524 the seismogram. In general, we can only use well k to do the comparison between the
525 timing of step in water level change and the arrival time of seismic waves. From the
526 occurrence time of water level changes and the arrival time of surface waves of well k
527 (Table 6), we find the co-seismic water level changes are attributed to the passage of
528 surface waves. From that, we may infer: in other wells the co-seismic water level
529 changes are attributed to the dynamic strain induced by the passage of teleseismic
530 waves, most probably surface waves, which have relatively larger amplitude of
531 oscillation, corresponding to relatively larger energy. The similar conclusion has been
532 proposed by Sil and Jeffrey (2006), West *et al.* (2005), and Chadha *et al.* (2008 II).
533 More precise estimation of the timing of the step could not be made because of the
534 low temporal resolution of the water level data. Obviously, there are geographic
535 position differences between the observation of seismograms and water levels, and
536 there are also some errors on the manual amplitude readings, both of which could
537 cause some influence on the analysis.

538 The PGV (peak ground velocity) of Fuxin (SNY station) is about 3.224 mm/s,
539 and that of well (k) (HEF station) is about 6.891 mm/s. Although the co-seismic water
540 level changes in Fuxin is smaller than that in well (k), since they are induced by
541 different mechanisms (co-seismic water level ($\Delta h=0.121\text{m}$) in Fuxin is induced by
542 increased permeability followed by a rapid redistribution of pore pressure, and
543 co-seismic water level ($\Delta h=-0.455\text{m}$) in well (k) is induced by dilatation), the ratio of
544 PGV should not directly related with the ratio of co-seismic water level changes in the

545 two wells.

546 There are aftershocks, and the one following the M_s 8.0 main shock (Chinese
547 time 14:27:59.5) is at 14:43:14.7 , it is about 15 minutes later, so it will not cause
548 disturbances on the main shock seismogram. What's more the after shocks are much
549 smaller (the magnitude of aftershocks are less than M_s 6.0) than the main shock, the
550 energy will decrease about 900 times, when the magnitude decrease 2, so the energy
551 of those aftershocks are much smaller, which are not large enough to induce the
552 variation of water level.

Conclusion

553 Together with the variation of Skempton's coefficient B , the change of pore
554 pressure and the inferred variation of effective pressure in each well, we can infer the
555 mechanism of the co-seismic water level changes. From the study we can conclude:
556 consolidation/dilatation induced by shaking of teleseismic waves, may account for the
557 mechanisms of those coseismic water level changes, for which the variation tendency
558 of the co-seismic water level, and the effective pressure keep the same (all increase or
559 all decrease), and most of those wells have relatively high permeabilities attributed to
560 the shales in the matrix rocks (based on the obtained 7 well logs in this study). While,
561 fracture clearing and increased permeability with a rapid redistribution of pore
562 pressure may be used to explain the other part of those coseismic water level changes,
563 for which the co-seismic water level, and the effective pressure change with
564 inconformity. Most of those wells stay in basins or hollows (well f, h, i and Fuxin),
565 this kind of terrain inclines to lead to heterogeneous pore pressure in close proximity.
566 Compared with the seismograms, the co-seismic water level changes are attributed to

567 the dynamic strain induced by the passage of seismic waves, most probably long
568 period surface waves. Our analysis is not conflict with any of those existing theories.
569 Although those water level changes happened in the intermediate and far fields, most
570 of those water levels present abrupt and obvious co-seismic changes owing to the
571 huge energy of M_s 8.0 Wenchuan earthquake.

572 Experiments of [Liu and Manga \(2009\)](#) apply time varying axial stresses
573 (confining pressure changes) whereas [Elkhoury et al. \(2011\)](#) applied time varying
574 fluid pressure differences (pore pressure changes) across their samples. Our study
575 complement the experiments of both of them, we discusse the change of effective
576 pressure ($P_{eff} = P_c - P_p$) in two ways: A) Pore pressure P_p keeps constant, the change
577 of effective pressure induced by the change of confining pressure P_c . B) Confining
578 pressure P_c keeps constant, the change of effective pressure induced by the change of
579 pore pressure P_p .

580 From the analysis of Fuxin well, we can see consolidation also can be incurred
581 by teleseismic waves. As discussed by [Liu and Manga \(2009\)](#): Dynamic strains cause
582 time varying fluid flow that can redistribute particles within fractures or porespace,
583 and can allow particles to move away from regions where they hold pore spaces open,
584 and are expected to accumulate and get trapped at the narrowest constrictions along
585 flow paths, and hence allow a consolidation (contraction) of the sample. The
586 mechanism analyzed in this paper are similar to the experiment results of [Liu and](#)
587 [Manga \(2009\)](#), and our in-situ analysis may complement the limitation of the initial
588 condition of their laboratory experiment.

589 Matrix rocks with shales always correspond to relatively high permeabilities, and
590 those wells are well connected with the other places outside the well, so there may be

591 little pressure differences between those wells and the places around them, and water
592 levels tend not to flow into or out of those wells even if the porosity/permeability
593 increased by the shaking of teleseismic waves. From our study we find most of the
594 co-seismic water level changes in those wells can be attributed to the change of
595 confining pressures. Consolidation/dilatation induced by shaking of teleseismic waves,
596 may account for the mechanism. According to the geological structures, all of those
597 wells stay in faults or in fracture intersections. Meanwhile, we find that the wells in
598 which the coseismic water level changes can be explained with “the enhanced
599 permeability with a rapid redistribution of pore pressure”, stay in basins or in hollows
600 (well f, h, i and Fuxin). The terrains of those wells incline to lead to heterogeneous
601 pore pressures in close proximities (possibly attributed to different altitudes).

602 In reality, some well aquifers are not porous and may be fractured, especially
603 those wells with shales in the matrix rocks, may display substantial anisotropy or a
604 fractured property rather than a porous property, however, we suspect that the
605 isotropic and homogeneous poroelastic theory we used here is the best available
606 approximation. The Skempton coefficients are very small for many wells, which may
607 be attributed to the value of the shear modulus G [see [Zhang and Huang \(2011\)](#), since
608 we lack the in-situ G values, we investigate the geology of each well and referred to
609 the <rock mass mechanism> ([Liu and Tang, 1998](#)), using the dynamic elastic modulus
610 and dynamic Poisson’s ratios to estimate the ranges of the dynamic shear modulus of
611 those matrix rocks (according to the formula $G = \frac{E}{2(1+\sigma)}$), and to choose the
612 approximate mean values ([Table 1](#))]. The shear modulus G and the undrained
613 Poisson’s ratio ν_u would change slightly after the shaking of seismic waves, and the
614 discussed “undrained” condition can hardly last for a long time, as long as the fluid

615 flow exists, the undrained condition will disrupt and be replaced by the drained
616 condition soon. We assume the results get from sandstone (Figure 3) can be applied to
617 all those bedrocks in those wells, however this is not very precise. As described by
618 Wang (1993) nonlinear compaction effects can be significant and they are not
619 incorporated in the linear theory presented here. Because the well aquifers are under
620 lithostatic pressures for a long time and withstand large numbers of seismic shaking,
621 the irreversible deformations and the nonlinear effects have been minimized (In the
622 laboratory experiment, in order to reduce the irreversible deformation and to minimize
623 the nonlinear effects, repeated pressure cycles are always applied on rock samples as
624 preconditions (Blocher *et al.*, 2009)). Discard all those ideal assumptions, things may
625 be different.

626 From our study we find lots of factors will influence the far-field co-seismic
627 water level changes, such as lithology, topography and geometry of the well. Later it
628 is necessary to calculate the transmissivity (permeability), so as to testify the
629 mechanisms. The commonly used permeability calculation [based on equations of
630 Hsieh *et al.* (1987)] is based on several parameters: the dimensionless storage
631 coefficient S , the radius of the screened or open portion of the well r_w , the radius of
632 the well casing r_c . Because we lack the logs for all those wells, it is hard to confirm
633 r_w or r_c for all of them. In our study we find the permeability increase in 4 wells
634 (well f, h, i and Fuxin), only well f and Fuxin well have the records of lithologic logs.
635 However, there are no direct records of r_w and r_c in <China earthquake monitoring
636 record series>, and it is hard to confirm r_w or r_c from the lithologic logs (Figure 6)
637 for the two wells. We have to give up the calculation of permeability in this paper,
638 alternatively, we use the phase lag between water level and tidal strain to

639 approximately estimate the variation of permeability before and after the earthquake.
640 Later, we may focus on 1—2 wells, which have detailed records of borehole datas,
641 water level, and seismogram, and then we may do analysis of the permeability,
642 together with the Skempton's coefficient B , so as to do comparison and to reveal the
643 mechanisms more deeply and clearly.

Data and Resources

644 Data used in this paper were collected using a classified network (Groundwater
645 Monitoring Network, GMN) of the China Earthquake Networks Center and cannot be
646 released to the public. We use the Mapseis software (Lu *et al.*, 2002) to calculate the
647 tidal strain data.

Acknowledgements. We thank professor Emily Brodsky, and postdoctor
Jinlai Hao for discussion and help on this paper. This research was supported by the
Natural Science Foundation of China (Grant No. 40925013 and 41040036). We
gratefully appreciate the valuable suggestions proposed by the anonymous reviewers.

Reference

- 648 Brodsky, E., E. Roeloffs, D. Woodcock, I. Gall, and M. Manga (2003). A mechanism
649 for sustained groundwater pressure changes induced by distant earthquake, *J.*
650 *Geophys. Res.* **108(B8)**, 2390.
- 651 Beresnev, I., W. Gaul, and R. D. Vigil (2011). Direct pore-level observation of
652 permeability increase in two-phase flow by shaking, *Geophys. Res. Lett.* **38**,
653 L20302.
- 654 Blocher G., G. Zimmermann and H. Milsch (2009). Impact of poroelastic response of
655 sandstone on geothermal power production, *Pure appl. geophys.* **166**,

656 1107–1123.

657 Bower, D. R., and K. C. Heaton (1978). Response of an aquifer near Ottawa to tidal
658 forcing and the Alaskan earthquake of 1964, *Can. J. Earth Sci.* **15**, 331–340.

659 Berryman, J. G., and H. F. Wang (2001). Dispersion in poroelastic systems, *Phys. Rev.*
660 *E* **64**, 011303.

661 Berryman, J. G. (1999). Origin of Gassmann’s equations, *Geophysics* **64**, 1627–1629.

662 Beaumont, C. and J. Berger (1975). An analysis of tidal strain observations from the
663 United States of America: I. The laterally homogeneous tide, *Bull. Seismol. Soc.*
664 *Am.* **65**, 1613–1629.

665 Bodvarsson, G. (1970). Confined fluids as strainmeters, *J. Geophys. Res.* **75(14)**,
666 49–56.

667 Chadha, R. K., H. J. Kuempel and M. Shekar (2008 I). Reservoir triggered seismicity
668 (RTS) and well water level response in the Koyna-Warna region, India.
669 *Tectonophysics* **456**, 94–102. nI

670 Chadha, R. K., C. Singh, and M. Shekar (2008 II). Transient changes in well-water
671 level in bore wells in western India due to the 2004 Mw 9.3 Sumatra
672 Earthquake, *Bull. Seismol. Soc. Am.* **98**, 2553–2558.

673 Cooper, H. H., Jr., J. D. Bredehoeft, I. S. Papadopoulos, and R. R. Bennett (1965). The
674 response of well-aquifer systems to seismic waves, *J. Geophys. Res.* **70**,
675 3915–3926.

676 Doan, M. L., E. E. Brodsky, R. Prioul and C. Signer (2006). Tidal analysis of
677 borehole pressure-A tutorial, *Schlumberger Research report*, P35.

678 Elkhoury, J. E., E. E. Brodsky and D. C. Agnew (2006). Seismic waves increase

679 permeability, *Nature* **411**, 1135–1138.

680 Elkhoury, J. E., A. Niemeijer, E. E. Brodsky, and C. Marone (2011). Laboratory
681 observations of permeability enhancement by fluid pressure oscillation of in situ
682 fractured rock, *J. Geophys. Res.* **116**, B02311.

683 Green, D. H. and H. F. Wang (1986). Fluid pressure response to undrained
684 compression in saturated sedimentary rock, *Geophysics* **51(4)**, 948–956.

685 Geballe, Z. M., C.-Y. Wang, and M. Manga (2011). A permeability-change model for
686 water-level changes triggered by teleseismic waves, *Geofluids* **11**, 302–308.

687 Gassmann, F. (1951). Über die elastizität poröser medien, *Vierteljahrsschrift der*
688 *Naturforschenden Gesellschaft in Zürich*, **96**, 1–23.

689 Huang, F.-Q. (2008). *Response of Wells in Groundwater Monitoring Network in*
690 *Chinese Mainland to Distant Large Earthquakes*, [Ph.D Dissertation] Institute
691 of Geophysics, China Earthquake Administration, Beijing, pp. 47, (in Chinese).

692 Hsieh, P., J. Bredehoeft, and J. Farr (1987). Determination of aquifer transmissivity
693 from earthtide analysis, *Water Resour. Res.* **23**, 1824–1832.

694 Khan, S. A., and H. G. Scherneck (2003). The M2 ocean tide loading wave in Alaska:
695 vertical and horizontal displacements, modelled and observed, *J. Geodesy* **77**,
696 117–127.

697 Kayen, R. E., E. Thompson, D. Minasian, R. E. S. Moss, B. D. Collins, N. Sitar, D.
698 Dreger, G. A. Carver (2004). Geotechnical reconnaissance of the 2002 Denali
699 Fault, Alaska, Earthquake. *Earthq. Spectra* **20**, 639–667.

700 King, C.-Y., S. Azuma, G. Igarashi, M. Ohno, H. Saito, and H. Wakita (1999).

701 Earthquake-related water-level changes at 16 closely clustered wells in Tono,
702 central Japan, *J. Geophys. Res.* **104**, 13,073–13,082.

703 Kano, Y., Yanagidani, T., (2006). Broadband hydroseismograms observed by closed
704 borehole wells in the Kamioka mine, central Japan: Response of pore pressure
705 to seismic waves from 0.05 to 2 Hz, *J. Geophys. Res.*, **111**, 1–11.

706 Linde, A. T., I. S. Sacks, M. J. S. Johnston, D. P. Hill, and R. G. Bilham (1994).
707 Increased pressure from rising bubbles as a mechanism for remotely triggered
708 seismicity, *Nature* **371**, 408–410.

709 Liu, W. Q., M. Manga (2009). Changes in permeability caused by dynamic stresses in
710 fractured sandstone, *Geophys. Res. Lett.* **36**, L20307.

711 Liu, C., M. W. Huang, and Y. B. Tsai (2006). Water level fluctuations induced by
712 ground motions of local and teleseismic earthquakes at two wells in Hualien,
713 eastern Taiwan, *TAO* **17**, 371–389.

714 Liu, L.B., E. Roeloffs, and X. Y. Zheng (1989). Seismically induced water level
715 fluctuations in the Wali Well, Beijing, China, *J. Geophys. Res.* **94**, 9453–9462.

716 Liu, Y. R., and H. M. Tang (1998). *Rock Mass Mechanics*, Press of China University
717 of Geosciences, Beijing, 214 pp.

718 Lu, Y. Z., S. L. Li, Z. H. Deng, H. W. Pan, S. Che, and Y. L. Li (2002). *Seismology*
719 *Analysis and Prediction System Based on GIS (Mapseis Software)*, Chengdu
720 Map Press, Chengdu, China, 232.

721 Magara, K. (1978). Compaction and fluid migration, *Practical petroleum geology*.
722 Amsterdam: Elsevier Scientific Publishing Company, 1–319.

- 723 Nur, A., and J. Byerlee (1971). An exact effective stress law for elastic deformation of
724 rocks with fluids, *J. Geophys. Res.* **76**, 6414–6419.
- 725 Rice, J. R., and M. P. Cleary (1976). Some basic stress diffusion solutions for fluid-
726 saturated elastic porous media with compressible constituents, *Rev. Geophys.* **14**,
727 227–241.
- 728 Roeloffs, E. A. (1996). Poroelastic techniques in the study of earthquakes-related
729 hydrologic phenomena, *Adv. Geophys.* **37**, 135–195.
- 730 Roeloffs, E. A. (1998). Persistent water level changes in a well near Parkfield,
731 California, due to local and distant earthquakes, *J. Geophys. Res.* **103**, 869–889.
- 732 Sil, S. (2006). Response of Alaskan wells to near and distant large earthquakes,
733 Master's Thesis, University of Alaska Fairbanks, pp. 8–11.
- 734 Sil, S. and J. T. Freymueller (2006). Well water level changes in Fairbanks, Alaska,
735 due to the great Sumatra-Andaman earthquake, *Earth Planets Space* **58**,
736 181–184.
- 737 Terzaghi, K. (1925). Principles in soil mechanics, III. Determination of the
738 permeability of clay, *Engineering News Record.* **95**, 832–836.
- 739 Theo, H. S., M. H. Jacques, and C. C. Stephen (2002). Estimation of the poroelastic
740 parameters of cortical bone, *J. Biomech.* **35**, 829–835.
- 741 Wang, H. F. (2000). *Theory of linear poroelasticity with application to geomechanics*
742 *and hydrogeology*. Princeton University Press, Princeton, pp. 5–6.
- 743 Wang, H. F. (1993). Quasi-static poroelastic parameters in rock and their geophysical
744 applications, *PAGEOPH* **141**, 269–286.
- 745 Wang, C.-Y. (2007). Liquefaction beyond the near field, *Seismol. Res. Lett.* **78**,

746 512–517.

747 Wang, C.-Y., Y. Chia, P. L. Wang and D. Dreger (2009). Role of S waves and Love
748 waves in coseismic permeability enhancement, *Geophys. Res. Lett.* **36**, L09404.

749 Wang, C.-Y., and M. Manga (2010). *Earthquakes and Water, Series: Lecture Notes in*
750 *Earth Sciences*. Springer Press, Berlin, pp. 18–24.

751 Wang, C.-Y., and Y. Chia (2008). Mechanism of water level changes during
752 earthquakes: Near field versus intermediate field, *Geophys. Res. Lett.* **35**,
753 L12402.

754 Walters, R. A. and D. G. Goring (2001). Ocean tides around New Zealand, *New Zeal.*
755 *J. Mar. Fresh.* **35**, 567–579.

756 Wu, H. Z., L. Y. Fu and H. K. Ge (2010). Quantitative analysis of basin-scale
757 heterogeneities using sonic-log data in the Yanchang Basin, *J. Geophys. Eng.* **7**,
758 41–50.

759 West, M., J. Sanchez, and S. McNutt (2005). Periodically-triggered seismicity at Mt.
760 Wrangell volcano following the Sumatr earthquake, *Science*, **308**,1144–1146.

761 Xue, Y.-Q. (1986). *Groundwater dynamics*. Geology Press, Beijing, pp. 16, (in
762 Chinese).

763 Zhang, Y., F. Q. Huang, and G. J. Lai (2009). Research on Skempton’s coefficient *B*
764 based on the observation of groundwater of Changping station, *Earthq. Sci.* **22**,
765 631–638.

766 Zhang, Y., and F. Q. Huang (2011). Mechanism of Different Coseismic Water-Level
767 Changes in Wells with Similar Epicentral Distances of Intermediate Field, *Bull.*

768 *Seismol. Soc. Am.* **101**, 1531–1541.

769 Zeng, R. S. (1984). *Solid Geophysics Introduction*, Press of Science, Beijing, pp. 9,
770 (in Chinese).

771 Zhao L. F., X. B. Xie, W. M. Wang, and Z. X. Yao (2008). Regional Seismic
772 Characteristics of the 9 October 2006 North Korean Nuclear Test, *Bull. Seismol.*
773 *Soc. Am.* **98**, 2571–2589.

Appendix: An approach to Skempton's coefficient B based on the poroelastic theory

774 Skempton's coefficient B is a significant pore-fluid parameter in poroelastic
775 theory. A poroelastic material consists of an elastic matrix containing interconnected
776 fluid saturated pores. Fluid saturated crust behaves as a poroelastic material to a good
777 degree of approximation.

778 [Rice and Cleary \(1976\)](#) summarized the following equations for a linearly elastic
779 isotropic porous medium (they are the building blocks of the poroelastic theory):

$$780 \quad 2G\varepsilon_{ij} = \sigma_{ij} - \frac{\nu}{1+\nu} \sigma_{kk} \delta_{ij} + \frac{3(\nu_u - \nu)}{B(1+\nu)(1+\nu_u)} p \delta_{ij}, \quad (\text{A1})$$

$$781 \quad m - m_0 = \frac{3\rho(\nu_u - \nu)(\sigma_{kk} + 3p/B)}{2GB(1+\nu)(1+\nu_u)}. \quad (\text{A2})$$

782 Here $m - m_0$ is the change of the fluid mass, ε_{ij} is the strain tensor, σ_{ij} is the stress
783 tensor, δ_{ij} is the Kronecker delta function, G is the shear modulus, ρ is the
784 density of the fluid, B is the Skempton's coefficient, p is the pore pressure, ν is
785 the Poisson's ratio, and ν_u is the "undrained" Poisson's ratio. [Rice and Cleary \(1976\)](#)
786 describe equation (A1) as a stress balance equation and equation (A2) as a mass
787 balance equation.

788 For the undrained condition, the poroelastic effect on the crust can be obtained
789 by putting $m - m_0 = 0$ in equation (A2) to obtain

$$790 \quad p = -B\sigma_{kk} / 3 \quad \text{or} \quad \Delta p = -B\Delta\sigma_{kk} / 3. \quad (\text{A3})$$

791 Equation (A3) indicates that, in the undrained condition, the change in fluid pressure
792 (Δp) is proportional to the change in mean stress ($\Delta\sigma_{kk} / 3$). This is the mechanism of
793 water level changes for poroelastic material. ($p = \rho gh$, where h is the water column
794 height, g is the acceleration due to gravity and ρ is the density of water).

795 According to equation (A3), Skempton's coefficient B can be qualitatively
796 defined: In the undrained condition, B is the ratio of the induced pore pressure divided
797 by the change in mean stress (Wang, 2000). B governs the magnitude of water-level
798 changes due to an applied stress because pore pressure is directly proportional to
799 water level. The value of B is always between 0 and 1. When B is 1, the applied stress
800 is completely transferred into changing pore pressure. When B equals 0, there is no
801 change in pore pressure after applying the stress. Thus a low value of B indicates the
802 stiff rock matrix that supports the load with low coupling to the fluid (Nur and
803 Byerlee, 1971). Laboratory studies indicate the value of B depends upon the fluid-
804 saturated pore volume of the sample (Wang, 2000).

805 Equation (A3) can be expressed in terms of tidal strain as well (Roeloffs, 1996):

$$806 \quad \Delta h = -\frac{2GB(1 + \nu_u)}{3\rho g(1 - 2\nu_u)} \Delta\varepsilon_t. \quad (\text{A4})$$

807 Equation (A4) shows that water level changes proportionally in a poroelastic material
808 under the influence of tidal strain (ε_t). Here, Δh is the change in height of water
809 level, and $\Delta\varepsilon_t$ is the corresponding tidal strain change (Sil, 2006).

810 From equation (A4) we obtain:

$$811 \quad B = -\frac{3\rho g(1-2\nu_u)}{2G(1+\nu_u)} \frac{\Delta h}{\Delta \varepsilon_t}. \quad (A5)$$

812 With equation (A5), we obtain the value of B with water level and tidal strain.
 813 However, the calculation must be on the strict premise of the undrained condition (the
 814 good correlation between the water level and the tidal strain) and should not be
 815 influenced by the other factors.

816 For the effect of the solid tide on the crust, when the wavelength of the tidal
 817 strain is much larger than the size of the aquifer, we can suppose the aquifer system is
 818 undrained (Huang, 2008). So we can suppose the effect of the M_2 wave in the crust
 819 can meet the undrained condition (Zhang *et al.*, 2009). In addition, those wells can
 820 record clear tidal strains and thus, because we calculate the phase lags between the
 821 water levels and the tidal strains are small, the wells can readily meet the undrained
 822 condition. In the M_2 - wave frequency domain, the water level and the tidal strain
 823 show a good correlation; Furthermore, the M_2 wave is hardly influenced by
 824 atmospheric pressure. We therefore distill the frequency domain of the M_2 wave
 825 from the water level and the tidal strain by using band-pass filter (the frequency of the
 826 M_2 wave is $2.23636 \times 10^{-5} \text{ HZ}$) to calculate the Skempton's coefficient B . By
 827 converting the frequency domain of the M_2 waves (obtained from the water level and
 828 the tidal strain), by inverse fast Fourier transform and adjusting their phases (using the
 829 least-square fit and putting the results into equation (A5)), we can finally derive B .
 830 (More details of the method are explained in Zhang *et al.*, 2009). All the Water-level
 831 observations come from the sensor of water level, while tidal strain data are calculated
 832 via Mapseis software (see Data and Resources section). One thing needs to be
 833 clarified: We haven't applied the static equations directly to relate pore pressure

834 changes to seismic waves. We use those static equations for the impact of the tidal
835 strain on the aquifer medium before and after the Wenchuan earthquake, so as to
836 obtain the pre- and post- earthquake Skempton's coefficient B (those two periods can
837 be recognized as two independent quasi-static processes), so the poroelastic static
838 equations can be applied.

Table 1. Basic information of well a ~ k.

Station	Epicentral Distance / km	Δh / m	Pre / Post-Earthquake B	Major Aquifer Lithology	G^*/Gpa	Phase Lag / hour	Well Diameter / mm	Well Depth / m	Range of P_{eff} / MPa	Geological Structure
(a) Xiaxian	465.9465	0.106	0.0123 / 0.0149	Biotite plagioclase gneiss and mild clay	40	L1=L2=0	559	170.5	0 ~ 3	north part of Zhongtiao mountain fault
(b) Mile	726.4589	0.579	0.0872 / 0.1103	Limestone	20	L1=L2=-6	127	614.4	3 ~ 5	Mile—Shizong fault
(c) Qinxianmanshui	983.8517	0.172	0.0557 / 0.0653	Three of Triassic sandstone	8	L1=L2=-2	134	240.05	0 ~ 3	Guocun basin, uplift of Taihang mountain fault block
(d) Xiaoyi	1062.0768	0.398	0.1493 / 0.186	P2 Sandstone	8	L1=L2=0	150	502.93	0 ~ 3	Jiaocheng fault
(e) Qixian	1152.6034	0.831	0.0906 / 0.0153	Limestone and shale (the Tertiary and Quaternary period loess and gravel)	20	L1=0 L2=-3	146	442.19	0 ~ 3	east part of Taiyuan basin
(f) Jurong	1750.2357	0.263	0.0472 / 0.0519	K2 Silicified sandstone and conglomerate	8	L1=L2=0	219	889.18	8 ~ 10	west of Maoshan mountain fault, near the top of the uplift of the buried hill of Jurong hollow
(g) Haiyuanganyanchi	606.402	-0.036	0.0407 / 0.0395	Q sandstone and conglomerate	8	L1=L2=0		306.73	0 ~ 3	west and south of Huashan mountain fault
(h) Guyuanzhenqi	638.7904	-0.026	0.0026 / 0.0047	Mediate and fine sand	8	L1=L2=0		255.74	0 ~ 3	compresso-shear basin, in the east and north part of Haiyuan fault
(i) Kaiyuan	805.4263	-0.155	0.0724 / 0.077	Triassic Falang formation limestone	20	L1=L2=0	273	224	0 ~ 3	south of Xiaojiang fault, east of arc structure top, in the northern part of the basin
(j) Meizhou	1345.951	-0.075	0.0873 / 0.0823	Quartzite	20	L1=L2=0		338.86	0 ~ 3	Heyuan - Shaowu and Chaoan - Meixian fracture intersection
(k) Chaohu	1587.6013	-0.455	0.091 / 0.0798	The Devonian quartz and limestone	20	L1=L2=0	168	331	0 ~ 3	East side of the Tanlu fault, Chuhe river major dislocation and Hefei—Dongguan fracture intersection.
Fuxin	1409.9764	0.121	0.5761 / 0.5145	Granite, basalt, andesite and clip tuff breccia	60	L1=-2 L2=-1		60.74	0 ~ 3	west and north of Fuxin fault basin

Epicentral Distances, Water Level Changes, Pre- and Post- Earthquake B Values, Major Lithology of Aquifers, Shear Modulus, Phase Lags, Well Diameters, Well Depths, Ranges of Effective Pressure and Geological Structures of those well-picked stations. L1 and L2 represent the pre- and post- earthquake phase lags (the lag of piezometer water level behind the tidal strain induced aquifer pressure) separately.

Shear modulus G^* see [Zhang and Huang \(2011\)](#).

Table 2. Coseismic water level changes induced by increased permeability.

Station	Δh / m	ΔB	ΔP_p / MPa	ΔP_{eff} / MPa	Well Depth / m	Range of P_{eff} / MPa
(f) Jurong	0.263	0.0047	0.0026	-0.0026	889.18	8 ~ 10
(h) Guyuanzhenqi	-0.026	0.0021	-0.0003	0.0003	255.74	0 ~ 3
(i) Kaiyuan	-0.155	0.0046	-0.0015	0.0015	224	0 ~ 3
Fuxin	0.121	-0.0616	0.0012	-0.0012	60.74	0 ~ 3

Water Level Changes Δh , Changes of B Value, Calculated Changes of Pore-Pressure ΔP_p , Inferred Changes of Effective Pressure ΔP_{eff} , Well Depths and Ranges of Effective Pressure of those wells.

Table 3. Coseismic water level changes induced by consolidation or dilatation incurred by shaking of teleseismic waves.

Station	$\Delta h / m$	ΔB	$\Delta P_p / MPa$	$\Delta P_{eff} / MPa$	Well Depth / m	Range of P_{eff} / MPa
(a) Xiaxian	0.106	0.0026	0.0010	0.001	170.5	0 ~ 3
(b) Mile	0.579	0.0231	0.0057	0.0057	614.4	3 ~ 5
(c) Qinxianmanshui	0.172	0.0096	0.0017	0.0017	240.05	0 ~ 3
(d) Xiaoyi	0.398	0.0367	0.0039	0.0039	520.93	0 ~ 3
(g) Haiyuanganyanchi	-0.036	-0.001	-0.0004	-0.0004	306.73	0 ~ 3
(j) Meizhou	-0.075	-0.005	-0.0007	-0.0007	338.86	0 ~ 3
(k) Chaohu	-0.455	-0.011	-0.0045	-0.0045	331	0 ~ 3

Water Level Changes Δh , Changes of B Value, Calculated Changes of Pore-Pressure ΔP_p , Inferred Changes of Effective Pressure ΔP_{eff} , Well Depths and Ranges of Effective Pressure of those wells.

Table 4. Sketch of mechanism analysis.

State	Confining pressure P_c	Pore pressure P_p	Effective pressure $P_p = P_c - P_p$	Coseismic water level change Δh	Deduced Mechanism
(a1)	↑	—	↑	↑	Consolidation
(a2)	↓	—	↓	↓	Dilatation
(b1)	—	↓	↑	↓	Increased permeability followed by a rapid redistribution of pore pressure (water level flow out of the well to a place with a relatively lower pore pressure)
(b2)	—	↑	↓	↑	Increased permeability followed by a rapid redistribution of pore pressure (water level flow into the well from a place with a relatively higher pore pressure)

“↑”depends increase, “↓”depends decrease, and “—”depends invariance.

Table 5. Well (e), an exception.

Station	Δh / m	ΔB	ΔP_p / MPa	ΔP_{eff} / MPa	Well Depth / m	Range of P_{eff} / MPa
(e) Qixian	0.831	-0.075	0.0081	-0.0081	422.19	0 ~ 3

Water Level Changes, Changes of B Value, Calculated Changes of Pore-Pressure ΔP_p ,

Inferred Changes of Effective Pressure ΔP_{eff} , Well Depths and Ranges of Effective

Pressure of well (e).

Table 6. Occurrence time of water level changes, arrival time of surface waves and peak ground velocities of well (k) and Fuxin well.

Well (water level) / Station (seismogram)	Occurrence time of water level change	Arrival time of surface wave	PGV (Z-component)
(k) Chaohu / HEF	14:32:00, May 12, 2008	14:31:29.5, May 12, 2008	6.891 mm/s
Fuxin (only hour data) / SNY	14:??, May 12, 2008	14:35:34.5, May 12, 2008	3.224 mm/s

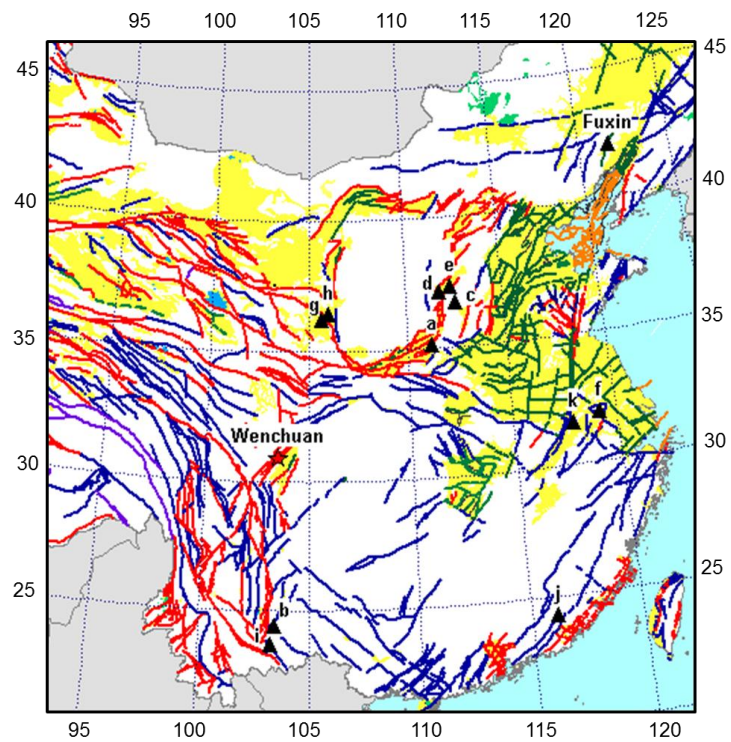


Figure 1. The selected 12 stations with distinct amplitude co-seismic water level changes during the Wenchuan earthquake in mainland China. The well numbers are in accordance with the numbers listed in [Table 1](#).

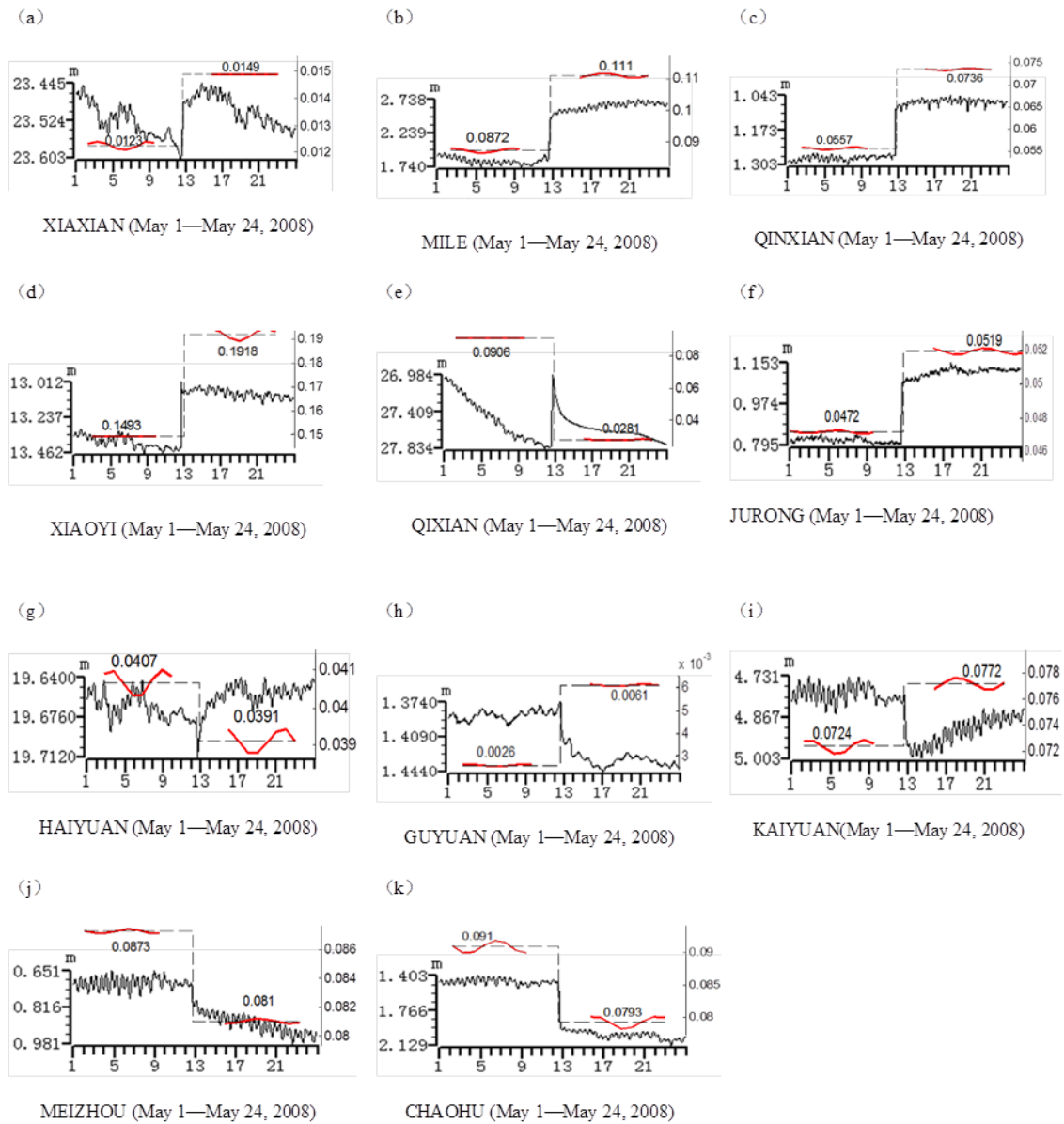


Figure 2. (a) Left y-coordinate: original water levels, the sequential number of y-coordinate depends on the type of the well, “sequential number increase from low to high” indicates an artesian well, the coordinate value means the height from the free water surface to the artesian discharge point or to the ground. “Sequential number decrease from low to high” indicates a non-artesian well, and the coordinate value means the depth from the free water surface to the ground. All the ascendant/descendent patterns in the picture indicate water level ascending/ descending. (b) Right y-coordinate: the calculated Skempton’s coefficient B . The dashed lines

indicate the mean B values, which are clearly shown in numbers. While the curves along the dashed lines indicate the continuous B values both pre- and post-earthquake.

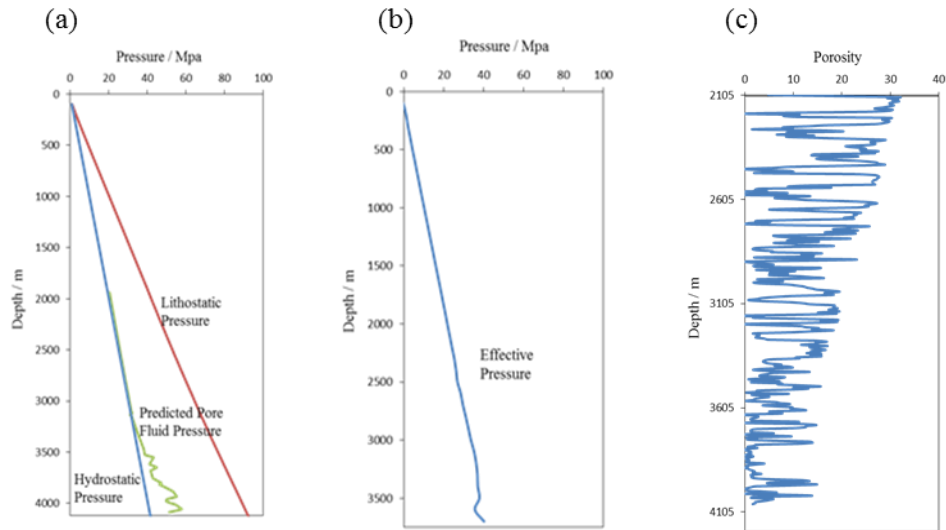
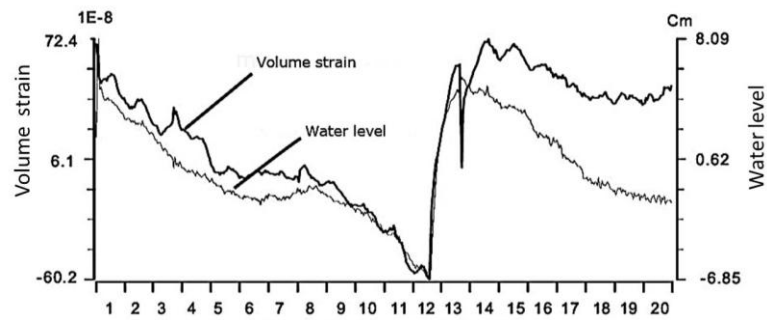


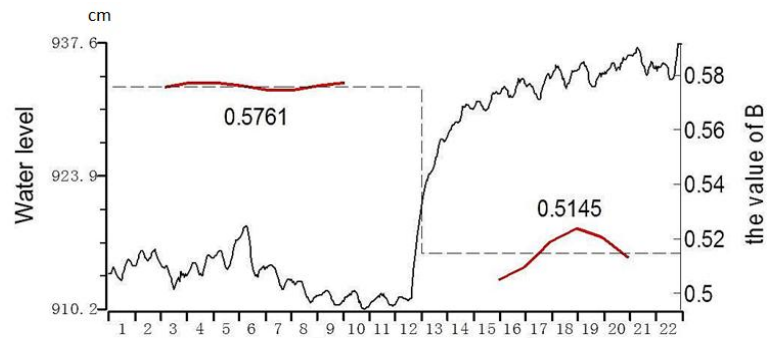
Figure 3. (a) Pressure section of W-1 well in Yanchang basin, the bedrock of W-1 well is sandstone. (b) Effective pressure section of W-1 well, we just show the depth above 3500m, so as to see the value in shallow depth more clearly. (c) Porosity section of W-1 well. The porosity records approximately starts from 2100 m, there are no records above this depth.

(a)



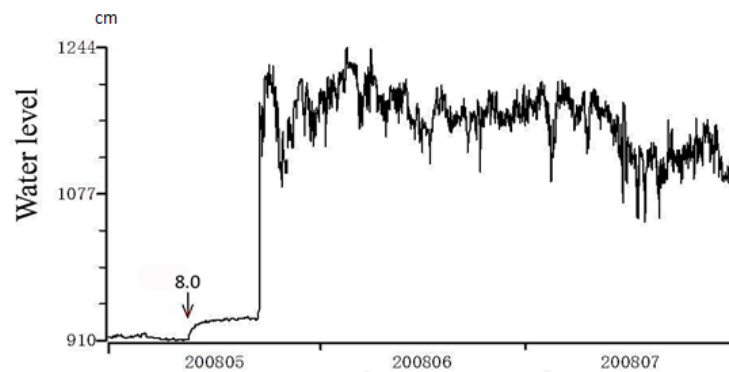
FUXIN (May 1—May 20, 2008)

(b)



FUXIN (May 1—May 22, 2008)

(c)



FUXIN (May 1—July 31, 2008)

Figure 4. Fuxin well (a) Corrected water level and volume strain after removing the influence of atmospheric pressure and tidal strain (based on the harmonic analysis method). In order to avoid the interfere of thunder, there is a power cut protection on

13 May, which is in accordance with the break point of the volume strain in the figure (Huang, 2008). (b) Original water level and the pre- and post- earthquak Skempton's coefficient B . (c) Original water level of Fuxin well form May, 2008 to July 2008.

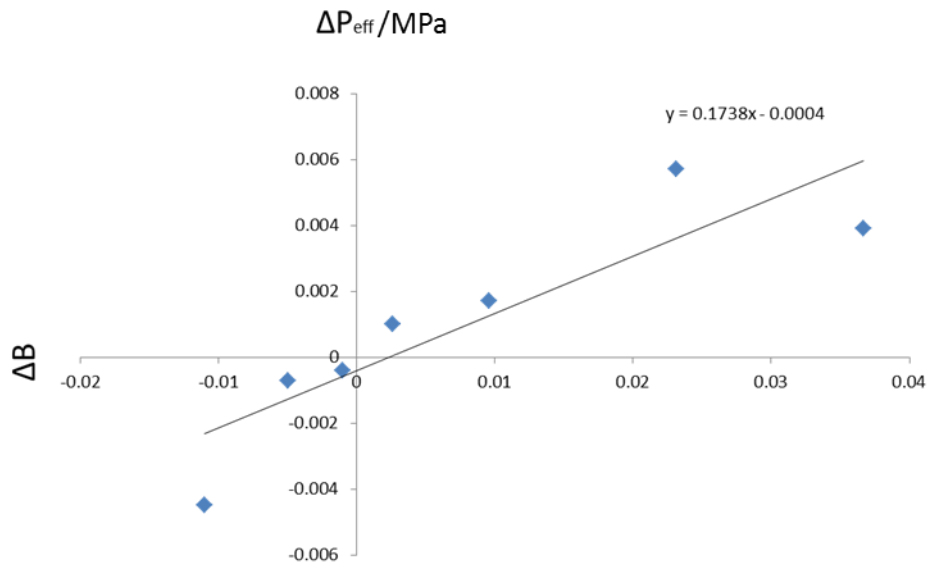
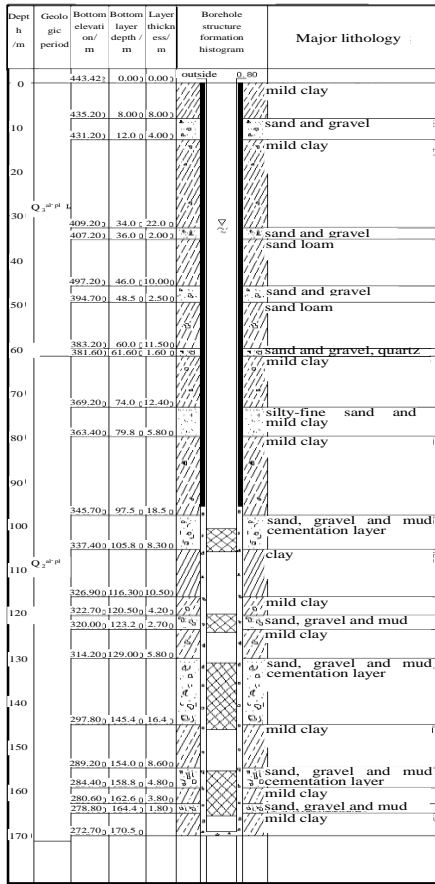
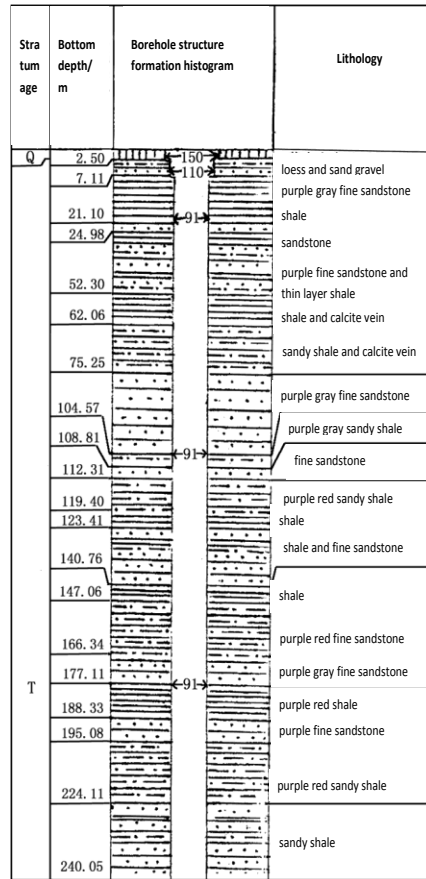


Figure 5. The relationship between the change of Skempton's coefficient B and the change of effective pressure P_{eff} of those wells of which the coseismic water level changes can be explained by the consolidation or dilatation caused by teleseismic waves.

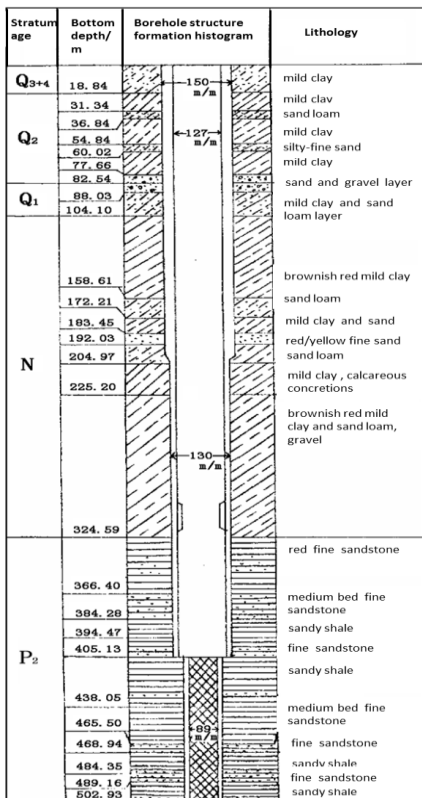
(a) Well (a)-Xiaxian



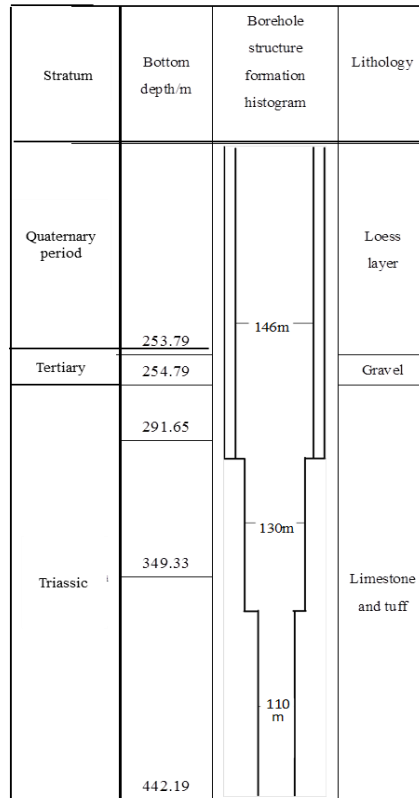
(b) Well (c)-Qinxianmanshui



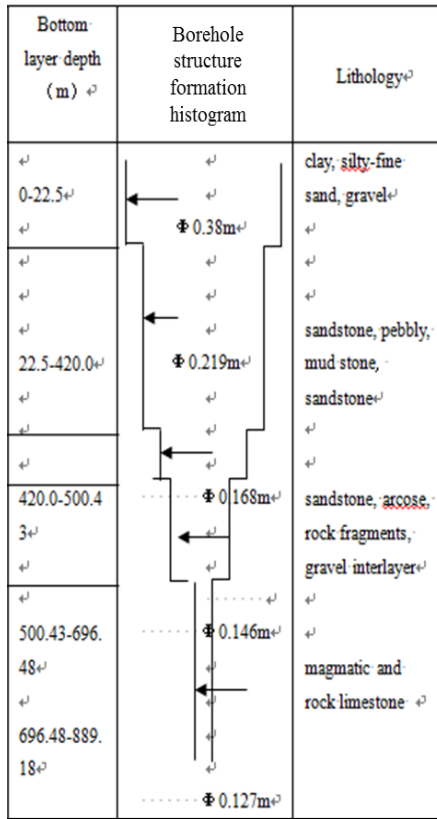
(c) Well (d)-Xiaoyi



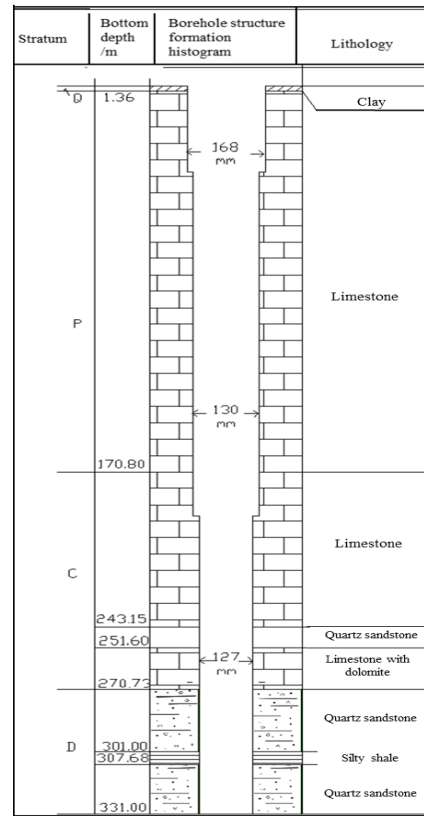
(d) Well (e)-Qixian



(e) Well (f)-Jurong



(f) Well (k)-Chaohu



(g) Fuxin

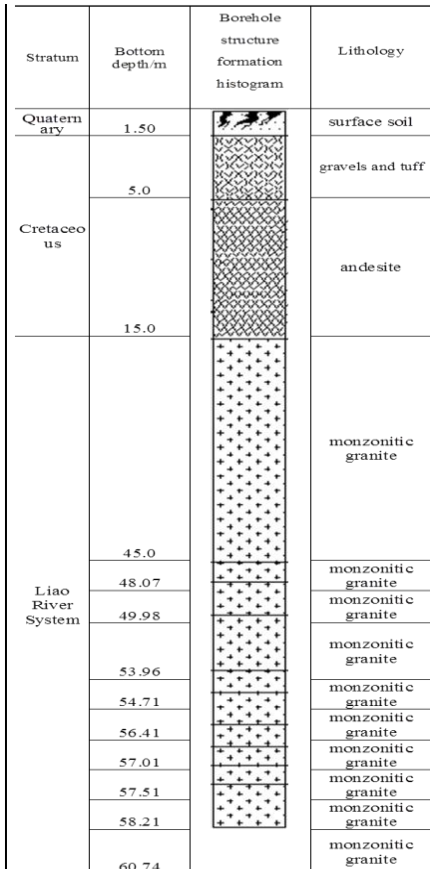
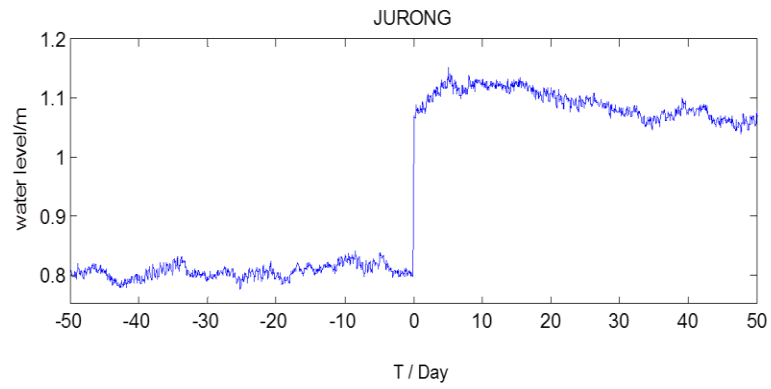


Figure 6. Lithologic logs (borehole structure histogram) of well (a), (c), (d), (e), (f), (k) and Fuxin.

(a)



(b)

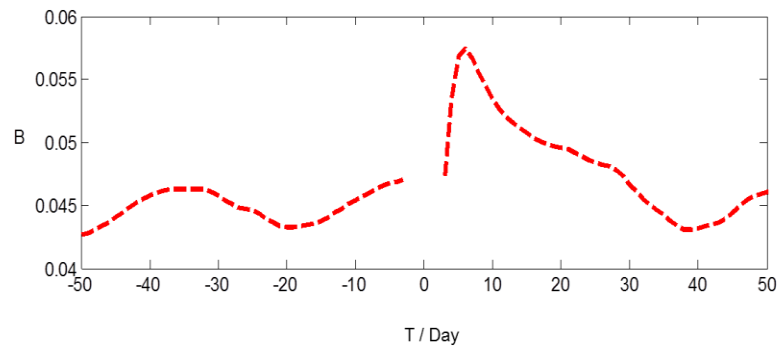


Figure 7. Jurong well (a) Original water level of Jurong station. (b) Continuous B value of Jurong station. (“0” depends the day when Wenchuan earthquake happened)

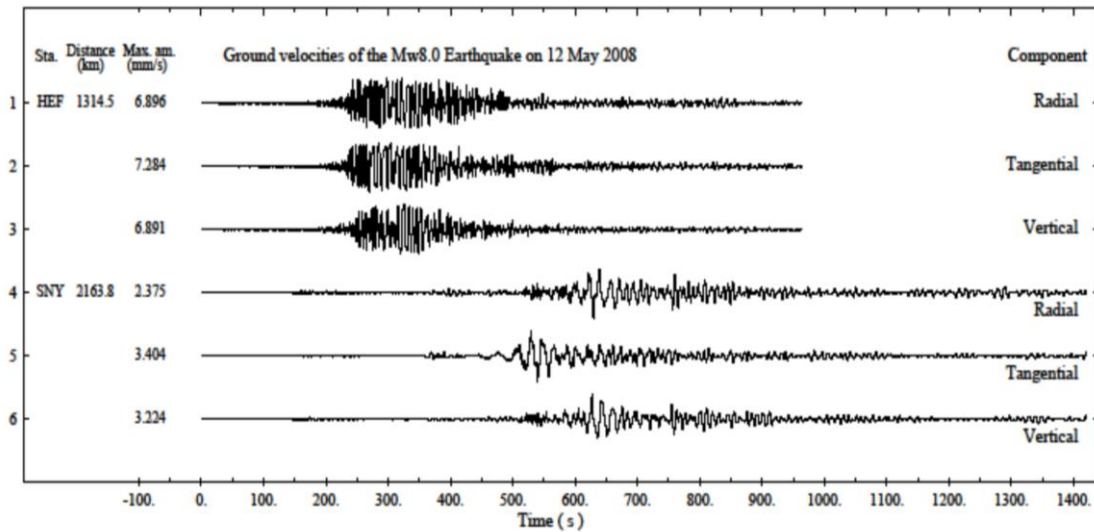


Figure 8. Seismograms of HEF and SNY national stations for the M_s 8.0 Wenchuan earthquake. The stations are ordered according to their epicentral distances. The station names and maximum amplitudes are listed on the left-hand side and are measured in millimetres per second. “0” is the time of Wenchuan earthquake: at 14:27:59.5, May12, 2008 (Chinese time). (This plotting pattern of seismograms are coined by [Zhao et al.\(2008\)](#)).

Bulletin of the Seismological Society of America

COPYRIGHT/PAGE CHARGES FORM

PLEASE FILL OUT AND RETURN THIS FORM (PLEASE USE YOUR OWN PAPER) WITH YOUR MANUSCRIPT TO THE EDITORIAL BOARD
OR FAX IT TO FAX NUMBER 601-693-7298

Manuscript Number: BSSA-D-

(2013) Bulletin for 2013 (submit manuscript)

Title: Studies of mechanisms for water level changes induced by teleseismic waves
Authors: Yan Zhang, Li-Yun Fu, Fuqiang Huang, Liang-feng Zhao, Yuchuan Ma, Bo Zhao and Baoxun Su.

COPYRIGHT

In accordance with Public Law 94-553, copyright in the articles herein shall be held by the author(s) and, if the author(s) is/are an employee(s) of the United States Government, the United States Government. In the event that the author(s) is/are an employee(s) of the United States Government, the author(s) shall retain the right to use all or part of the article in future works of their own. In addition, the author(s) affirm that the article has not been copyrighted and that it is not being submitted for publication elsewhere.

To be signed by at least one of the authors (who agrees to inform the editor, if any, of, to the extent of "work made for hire," by the employer.

Authorized Signature for Copyright

Print Name (if not author)

August 4th, 2013

PUBLICATION CHARGES

The Seismological Society of America requests that institutions supporting research share in the cost of publishing the results of that research. The Editor has the discretion of waiving page charges for authors who do not have institutional support. Contact your editor available at <http://www.seismosoc.org/publications/issue/authors/boas-page-charges.php>

Color options. Color figures can be published in (1) color both online and in print, (2) color online and in print, (3) color online and gray scale in print. Online color is free; authors will be charged for color in print. You have three online options for all of your figures within a paper: that is, you cannot choose option (1) for one color figure and option (2) for another color figure. You cannot submit two versions of the same figure, one for color and one in gray scale. You are responsible for ensuring that color figures are understandable when converted to gray scale, and that text, references and captions are appropriate for both online and print versions. **Color figures must be submitted before the paper is accepted for publication.** If color figures are changed to gray scale after acceptance of the paper, there will be a delay in publication while the paper undergoes further review by the Editorial Board.

An guidelines are at <http://www.seismosoc.org/publications/issue/authors/boas-authors/submitting-color.php>

Will page charges be paid? Check one:

BOTH PAGE CHARGES AND COLOR CHARGES WILL BE PAID, and all figures for this paper will be color online and in print. This option requires full payment of page and color charges. *Before choosing this option, please verify that there is funding to support color in print.* See <http://www.seismosoc.org/publications/issue/authors/boas-page-charges.php> for current rates.

ONLY PAGE CHARGES WILL BE PAID, and all figures for this paper will be gray scale in print. Color figures will only be color online.

A WAIVER OF PAGE CHARGES IS REQUESTED, and all figures will be gray scale in print. A color figure, if any, will be color online.

Send Invoice To:

If your paper is accepted for publication, SSA requires that you fill out and return (see <http://www.seismosoc.org/publications/issue/authors/boas-authors/submitting-color.php>)

Questions regarding billing should be directed to the SSA Business Office
Suite 701, Plaza Professional Building, 1141 Irvine, CA 92614, USA. Phone: 714-327-5000, Fax: 714-327-5001